

Company Name -

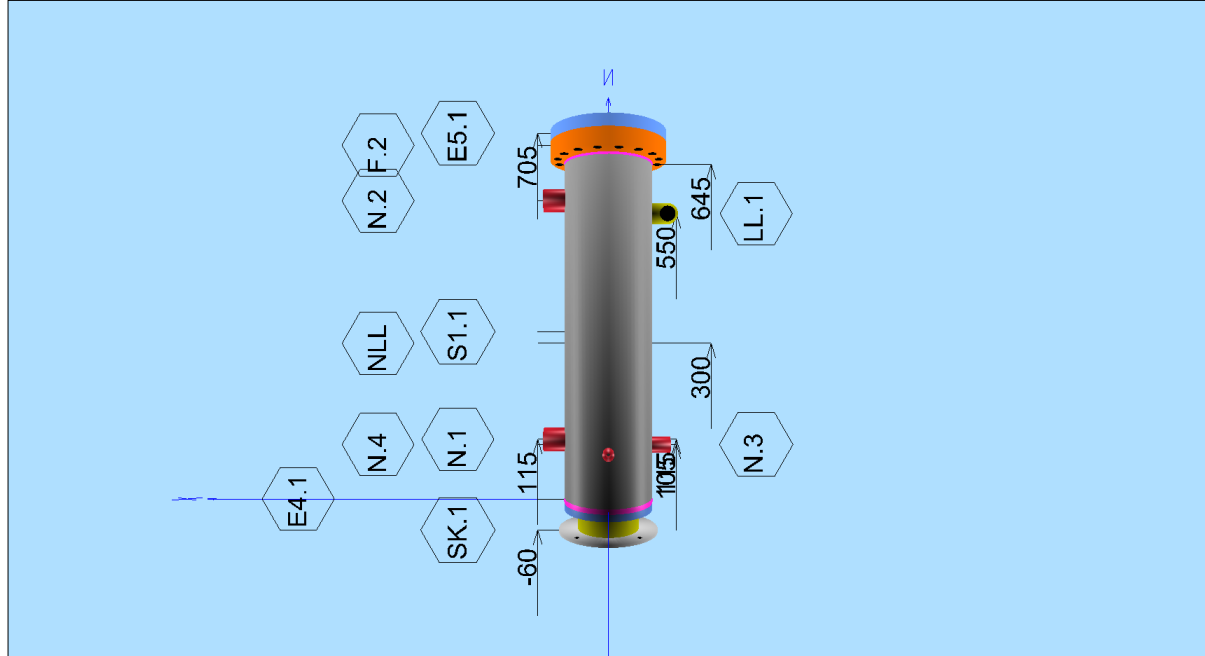
Client :GÜVEN SOGUTMA

Vessel Tag No.:OS.C.130B.35.1

Visual Vessel Design by Hexagon PPM,Ver:20.0 Operator : Rev.:A

(0) Drawing

3D View of Vessel (alter by using the Save User Specified View command)



Design Data & Process Information

Description	Units	Design Data
Process Card		General Design Data
Design Code & Specifications		EN13445 TG = 3b
Internal Design Pressure (MPa)	MPa	13
External Design Pressure (MPa)	MPa	
Hydrotest Pressure (MPa)	MPa	
Maximum Design Temperature (°C)	°C	130
Minimum Design Temperature (°C)	°C	-20
Operating Temperature (°C)	°C	
Corrosion Allowance (mm)	mm	0.5
Content of Vessel		
Specific Density of Oper.Liq		1.2
Normal Liquid Level NLL (mm)	mm	300

Test Pressure

TEST PRESSURE OF VESSEL - NEW & COLD - VERTICAL

Design Pressure..... : 13.000 MPa

Design Temperature..... : 130.0 C

ID	Description	Pdesign	PtMax	PtMin	Wat.Head	PtTop	PtTopMax
E4.1	Welded Flat End-Flat End	13.004	27.502	16.496	0.007	16.496	27.495
E5.1	Bolted Flat End-Cover Flange	13.000	27.192	16.496	0.000	16.496	27.192
F.2	RT - Flange-Shell Flange	13.000	22.460	16.496	0.001	16.496	22.459
N.1	Nozzle,Seamless Pipe-Inlet	13.002	24.194	NA	0.007	NA	24.187
N.2	Nozzle,Seamless Pipe-Outlet	13.000	24.194	NA	0.002	NA	24.191
N.3	Nozzle,Seamless Pipe-Safety Valve	13.002	25.054	NA	0.005	NA	25.049

Company Name -

Client :GÜVEN SOGUTMA

Vessel Tag No.:OS.C.130B.35.1

Visual Vessel Design by Hexagon PPM,Ver:20.0 Operator : Rev.:A

ID	Description	Pdesign	PtMax	PtMin	Wat.Head	PtTop	PtTopMax
N.4	Nozzle,Seamless Pipe-Oil Return	13.002	25.745	NA	0.006	NA	25.739
S1.1	Cylindrical Shell-Shell	13.004	26.605	16.370	0.007	16.370	26.598

PtReq = MAX(MIN(PtTop), 1.43*p)= 18.5900 MPa (EN13445-5, 10.2.3.3.1-1 & 2)

HYDRO-TEST

REQUIRED TEST PRESSURE AT TOP OF VESSEL PtReq(Hydro Test): 18.5900 MPa
MAXIMUM TEST PRESSURE AT TOP OF VESSEL PtLim(Hydro Test): 22.4594 MPa

PNEUMATIC TEST

REQUIRED TEST PRESSURE AT TOP OF VESSEL PtReq(Pneumatic Test) ..: 18.5937 MPa
MAXIMUM TEST PRESSURE AT TOP OF VESSEL PtLim(Pneumatic Test) ...: 22.4600 MPa

Note : Other components may limit Ptlim than the ones checked above.

TEST PRESSURE OF VESSEL - NEW & COLD - HORIZONTAL

Design Pressure.....: 13.000 MPa

Design Temperature.....: 130.0 C

ID	Description	Pdesign	PtMax	PtMin	Wat.Head	PtTop	PtTopMax
E4.1	Welded Flat End-Flat End	13.004	27.502	16.496	0.001	16.496	27.500
E5.1	Bolted Flat End-Cover Flange	13.000	27.192	16.496	0.003	16.496	27.190
F.2	RT - Flange-Shell Flange	13.000	22.460	16.496	0.002	16.496	22.458
N.1	Nozzle,Seamless Pipe-Inlet	13.002	24.194	NA	0.001	NA	24.193
N.2	Nozzle,Seamless Pipe-Outlet	13.000	24.194	NA	0.001	NA	24.193
N.3	Nozzle,Seamless Pipe-Safety Valve	13.002	25.054	NA	0.003	NA	25.051
N.4	Nozzle,Seamless Pipe-Oil Return	13.002	25.745	NA	0.002	NA	25.743
S1.1	Cylindrical Shell-Shell	13.004	26.605	16.370	0.002	16.370	26.603

PtReq = MAX(MIN(PtTop), 1.43*p)= 18.5900 MPa (EN13445-5, 10.2.3.3.1-1 & 2)

PhydOper = 0.0037 MPa PhydTest = 0.0028 MPa

PtReq = PtReq + (PhydOper-PhydTest) (EN13445-5, 10.2.3.3.1-3)

HYDRO-TEST

REQUIRED TEST PRESSURE AT TOP OF VESSEL PtReq(Hydro Test): 18.5910 MPa
MAXIMUM TEST PRESSURE AT TOP OF VESSEL PtLim(Hydro Test): 22.4577 MPa

PNEUMATIC TEST

REQUIRED TEST PRESSURE AT TOP OF VESSEL PtReq(Pneumatic Test) ..: 18.5947 MPa
MAXIMUM TEST PRESSURE AT TOP OF VESSEL PtLim(Pneumatic Test) ...: 22.4600 MPa

Note : Other components may limit Ptlim than the ones checked above.

NOMENCLATURE:

Pdesign- is the design pressure including liquid head at the part under consideration.

PtMax - is the maximum allowed test pressure determined at the part under consideration.

PtMin - is the required test pressure determined at the part under consideration.

Wat.Head - is the water head during hydrotesting at the part under consideration.

PtBot - is the required test pressure at bottom of the vessel, for the part under consideration.

PtTop - is the required test pressure at top of the vessel, for the part under consideration.

PtTopMax - is the maximum test pressure allowed at top of the vessel, for the part under consideration.

PtReq - is the required minimum test pressure (minimum value of PtTop) at top of vessel for the listed components.

PtLim - is the maximum allowed test pressure (minimum value for PtTopMax) at top of vessel for the listed components.

EN13445-5 10.2.3.3.8 Pressure of vessels under test shall be gradually increased to a value of approximately 50 % of the specified test pressure, thereafter the pressure shall be increased in stages of approximately 10 % of the specified test pressure until this is reached. The required test pressure shall be maintained for

Company Name -

Client :GÜVEN SOGUTMA

Vessel Tag No.:OS.C.130B.35.1

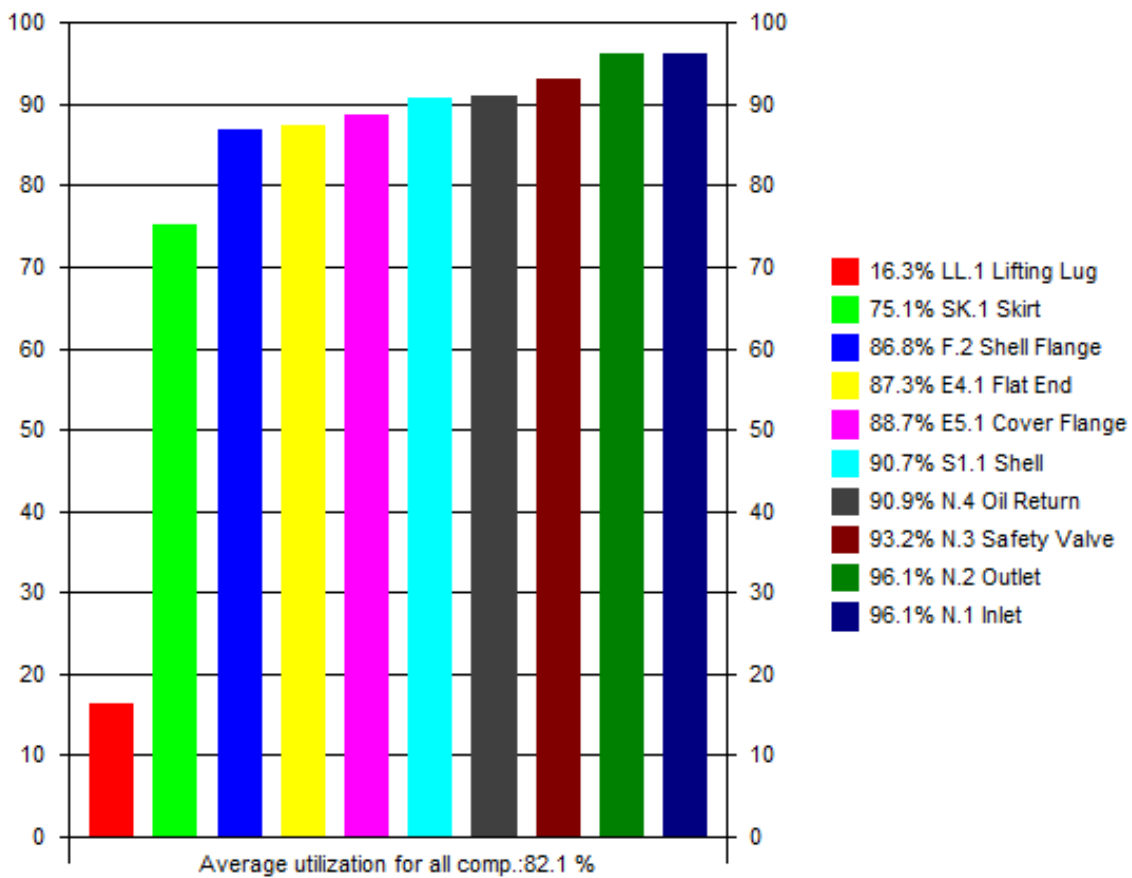
Visual Vessel Design by Hexagon PPM,Ver:20.0 Operator : Rev.:A

not less than 30 min. At no stage shall the vessel be approached for close examination until the pressure has been positively reduced by at least 10 % to a level lower than that previously attained. The pressure shall be maintained at the specified close examination level for a sufficient length of time to permit a visual inspection to be made of all surfaces and joints.

Utilization Chart

Utilization Chart

MPONENTS UTILIZATION CHART - Client :GÜVEN SOGUTMA Vessel Tag No.:OS.C.130B.3



Maximum Utilization of 96.1% for Component N.2 Outlet - VVD by Hexagon PPM, Ver:20.0

Company Name -

Client :GÜVEN SOGUTMA

Vessel Tag No.:OS.C.130B.35.1

Visual Vessel Design by Hexagon PPM,Ver:20.0 Operator : Rev.:A

EN13445:2014 Issue 5:2018+A5 - 7.4.2 CYLINDRICAL SHELL

S1.1 Shell

14 Jan. 2021 08:36

INPUT DATA

COMPONENT ATTACHMENT/LOCATION

GENERAL DESIGN DATA

PRESSURE LOADING: Design Component for Internal Pressure Only

PROCESS CARD:

General Design Data : Temp= 100°C, P=13.0000 MPa, c=0.5 mm, Pext=0.0000 MPa

SPECIFIC DENSITY OF OPERATING LIQUID.....:SG 1.2000

LIQUID HEAD.....:LH 300.00 mm

SHELL DATA

CYLINDER FABRICATION: Welded Pipe

WELD JOINT COEFFICIENT: Testing Group 1 (z=1.0)

NEGATIVE TOLERANCE: Negative tolerance specified in % of nominal thickness

EN 10217-3:2002/A1:05, 1.0565 P355NH welded tube, HT:N THK<=20mm 100'C

Rm=490 Rp=355 Rpt=304 f=202.67 f20=204.17 ftest=338.1 E=206067(N/mm2) ro=7.85

OUTSIDE DIAMETER OF SHELL.....:De 168.30 mm

LENGTH OF CYLINDRICAL PART OF SHELL.....:Lcyl 645.00 mm

NOMINAL WALL THICKNESS (uncorroded).....:en 7.1000 mm

NEGATIVE TOLERANCE/THINNING ALLOWANCE.....: 10.00 %

Calculate minimum shell thickness due to internal pressure at different elevations with steps of 1000 mm.: NO

Split shell into several shell courses and include welding information: NO

WELDING REQUIREMENTS TO EN 1708-1:2010

Comment(Optional):

Type of welded connection: Not Applicable

CALCULATION DATA

7.4.2 - CYLINDRICAL SHELLS UNDER INTERNAL PRESSURE

Required Minimum Shell Thickness Excl.Allow. emin :

$$emin = De * P / (2 * f * z + P) \quad (7.4-2)$$
$$=168.3*13./(2*202.67*1+13.)= 5.2313 \text{ mm}$$

Required Minimum Shell Thickness Incl.Allow. :

$$emina = emin + c + NegDev =5.23+0.5+0.71= 6.4413 \text{ mm}$$

Analysis Thickness

$$ea = en - c - NegDev =7.1-0.5-0.71= 5.8900 \text{ mm}$$

»7.4.1 Cond.of Applicability $emin/De=0.0311 \leq 0.16$ » OK«

Internal Pressure $emina=6.44 \leq en=7.1$ [mm]	90.7%	OK
---	-------	----

MAXIMUM ALLOWABLE WORKING PRESSURE MAWP :

Inside Diameter of Shell

$$Di = De - 2 * ea =168.3-2*5.89= 156.52 \text{ mm}$$

Mean Diameter of Shell

$$Dm = (De + Di) / 2 =(168.3+156.52)/2= 162.41 \text{ mm}$$

MAWP HOT & CORR. (Corroded condition at design temp.)

$$MAWPHC = 2 * f * z * ea / Dm =2*202.67*1*5.89/162.41= 14.70 \text{ MPa}$$

MAWP NEW & COLD (Uncorroded condition at ambient temp.)

$$MAWPNC = 2 * f20 * z * (ea + c) / Dm$$
$$=2*204.17*1*(5.89+0.5)/162.41= 16.07 \text{ MPa}$$

MAX TEST PRESSURE (Uncorroded cond.at ambient temp.)

Ptmax = 2 * ftest * ztest * (ea + c) / Dm

$$=2*338.1*1*(5.89+0.5)/162.41= 26.60 \text{ MPa}$$

Company Name -

Client :GÜVEN SOGUTMA

Vessel Tag No.:OS.C.130B.35.1

Visual Vessel Design by Hexagon PPM,Ver:20.0 Operator : Rev.:A

EN13445:2014 Issue 5:2018+A5 - 7.4.2 CYLINDRICAL SHELL

S1.1 Shell

14 Jan. 2021 08:36

EN13445-5;10.2.3.3 REQUIRED MIN.HYDROSTATIC TEST PRESSURE:Ptmin

NEW AT AMBIENT TEMP. FOR TEST GROUPS 1, 2 and 3

$$Ptmin = 1.25 * Pd * f20 / f = 1.25 * 13 * 204.17 / 202.67 =$$

16.37 MPa

$$Ptmin = 1.43 * Pd = 1.43 * 13 =$$

18.59 MPa

Test Pressure Ptmin=18.59 <= Pmax=26.6[MPa]**69.8%****OK**

MAXIMUM DIAMETER OF UNREINFORCED OPENING IN SHELL

Inside Radius of Shell

$$ris = Di / 2 (9.5-3) = 156.52 / 2 =$$

78.26 mm

Length of Shell Contributing to Reinforcement

$$Is = Sqr((2 * ris + ea) * ea)$$

(9.5-2)

$$= Sqr((2 * 78.26 + 5.89) * 5.89) =$$

30.93 mm

Maximum Diameter of Unreinforced Opening in Shell Checked to Rules in Section 9

$$dmax1 = MIN(0.5 * Di, (ea * Is * (f - 0.5 * P) / (P - ris * Is)) / (0.5 * ris + 0.5 * ea))$$

(9.5-7,22,23)

$$= MIN(0.5 * 156.52, (5.89 * 30.93 * (202.67 - 0.5 * 13.) / 13. - 78.26 * 30.93) / (0.5 * 78.26 + 0.5 * 5.89)) =$$

7.7885 mm

Maximum diameter of Opening Not Requiring Reinforcement Check

$$dmax2 = 0.15 * Sqr((2 * ris + ea) * ea)$$

(9.5-18)

$$= 0.15 * Sqr((2 * 78.26 + 5.89) * 5.89) =$$

4.6393 mm

Maximum Diameter of Unreinforced Opening

$$dmax = MAX(dmax1, dmax2) = MAX(7.79, 4.64) =$$

7.7885 mm

CALCULATION SUMMARY

7.4.2 - CYLINDRICAL SHELLS UNDER INTERNAL PRESSURE

Required Minimum Shell Thickness Excl.Allow. emin :

$$emin = De * P / (2 * f * z + P)$$

(7.4-2)

$$= 168.3 * 13. / (2 * 202.67 * 1 + 13.) =$$

5.2313 mm

Required Minimum Shell Thickness Incl.Allow. :

$$emina = emin + c + NegDev = 5.23 + 0.5 + 0.71 =$$

6.4413 mm

Internal Pressure emina=6.44 <= en=7.1[mm]**90.7%****OK**

MAX TEST PRESSURE (Uncorroded cond.at ambient temp.)

$$Ptmax = 2 * ftest * ztest * (ea + c) / Dm$$

$$= 2 * 338.1 * 1 * (5.89 + 0.5) / 162.41 =$$

26.60 MPa

EN13445-5;10.2.3.3 REQUIRED MIN.HYDROSTATIC TEST PRESSURE:Ptmin

NEW AT AMBIENT TEMP. FOR TEST GROUPS 1, 2 and 3

$$Ptmin = 1.25 * Pd * f20 / f = 1.25 * 13 * 204.17 / 202.67 =$$

16.37 MPa

$$Ptmin = 1.43 * Pd = 1.43 * 13 =$$

18.59 MPa

Test Pressure Ptmin=18.59 <= Pmax=26.6[MPa]**69.8%****OK**

MAXIMUM DIAMETER OF UNREINFORCED OPENING IN SHELL

Maximum Diameter of Unreinforced Opening

$$dmax = MAX(dmax1, dmax2) = MAX(7.79, 4.64) =$$

7.7885 mm

Volume:0.0124 m3 Weight:18.2 kg (SG= 7.85)

Company Name -

Client :GÜVEN SOGUTMA

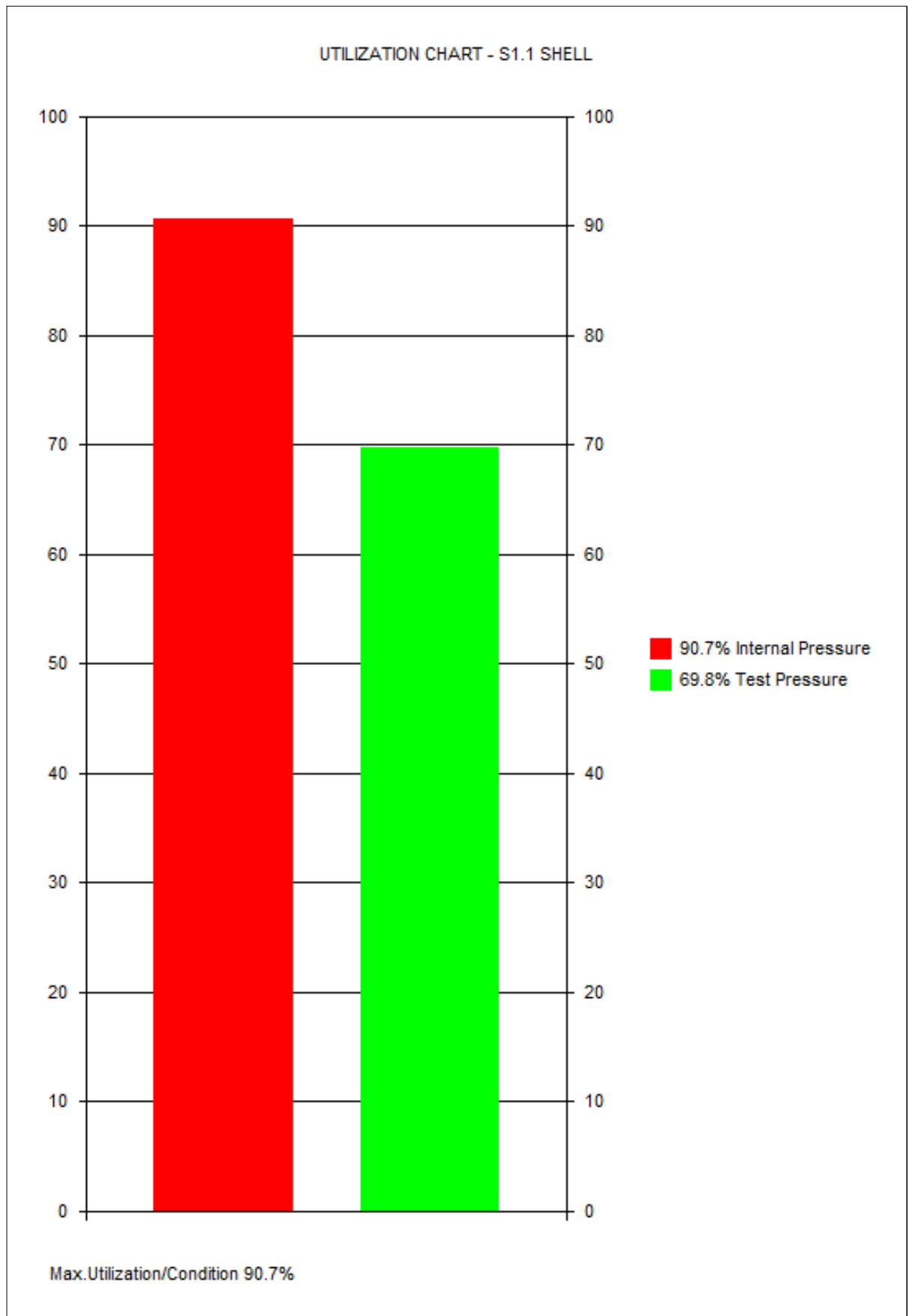
Vessel Tag No.:OS.C.130B.35.1

Visual Vessel Design by Hexagon PPM,Ver:20.0 Operator : Rev.:A

EN13445:2014 Issue 5:2018+A5 - 7.4.2 CYLINDRICAL SHELL

S1.1 Shell

14 Jan. 2021 08:36



Company Name -

Client :GÜVEN SOGUTMA

Vessel Tag No.:OS.C.130B.35.1

Visual Vessel Design by Hexagon PPM,Ver:20.0 Operator : Rev.:A

EN13445:2014 Issue 5:2018+A5 - 10.4 WELDED CIRCULAR FLAT END

E4.1 Flat End 09 Feb. 2021 14:41 ConnID:S1.1

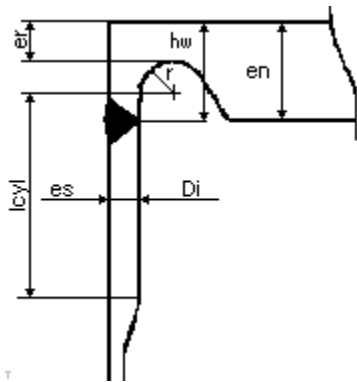
INPUT DATA

COMPONENT ATTACHMENT/LOCATION

Attachment: S1.1 Cylindrical Shell Shell
Location: Along z-axis zo= 0

GENERAL DESIGN DATA

PROCESS CARD: General Design Data : Temp= 100°C, P=13.0000 MPa, c=0.5 mm
SPECIFIC DENSITY OF OPERATING LIQUID.....:SG 1.2000
LIQUID HEAD.....:LH 300.00 mm
Check Deflection of Cover to TEMA RCB-9-21(multipass units): NO
Shape of Cover: Circular
Stayed Flat Plate to section 20.2: NO
Circular flat ends with radial reinforcement ribs to section 21: NO



TYPE OF FLAT WELDED END: Flat end with relief groove

DATA FOR CYLINDRICAL SHELL SECTION

CYLINDER DIAMETER: Base Design on Cylinder Outside Diameter
OUTSIDE DIAMETER OF SHELL.....:De 168.30 mm
NOMINAL WALL THICKNESS (uncorroded).....:esn 7.1000 mm
NEGATIVE TOLERANCE/THINNING ALLOWANCE.....:th 0.7100 mm
LENGTH OF CYLINDRICAL PART OF END.....:Lc 645.00 mm
EN 10217-3:2002/A1:05, 1.0565 P355NH welded tube, HT:N THK<=20mm 100'C
Rm=490 Rp=355 Rpt=304 fs=202.67 fs20=204.17 fstest=338.1 E=206067(N/mm2) ro=7.85

DATA FOR END/BLIND FLANGE

NOMINAL THICKNESS OF HEAD/END (uncorroded).....:en 18.00 mm
NOMINAL THK.OF END UNDER RELIEF GROOVE(uncorroded)..:ern 13.00 mm
INSIDE RADIUS OF RELIEF GROOVE.....:rd 5.0000 mm
Exclude option in 10.4.4.4, no fatigue analysis will be performed.: NO
EN 10028-2:2017, 1.0473 P355GH plate and strip, HT:N THK<=40mm 100'C
Rm=510 Rp=345 Rpt=314 f=209.33 f20=212.5 fstest=328.57 E=206067(N/mm2) ro=7.85

WELDING REQUIREMENTS TO EN 1708-1:2010

Comment(Optional):
Type of welded connection: Not Applicable

CALCULATION DATA

$$Di = De - 2 * (esn - c) = 168.3 - 2 * (7.1 - 0.5) = \underline{155.10 \text{ mm}}$$

Minimum thk.of cylinder section due to pressure only, esmin

$$esmin = P * Di / (2 * fs - P) \\ = 13. * 155.1 / (2 * 202.67 - 13.) = \underline{5.1406 \text{ mm}}$$

Length of cylindrical shell contributing to junction strength Icycl

$$Icycl = \text{Sqr}((Di + es) * es) (10.4-2) = \text{Sqr}((155.1 + 5.89) * 5.89) = \underline{30.79 \text{ mm}}$$

Analysis Thickness of a Uniform Cyl. Shell es

$$es = (esn - c - th) * \text{MIN}(Lc, Icycl) / Icycl$$

Company Name -

Client :GÜVEN SOGUTMA

Vessel Tag No.:OS.C.130B.35.1

Visual Vessel Design by Hexagon PPM,Ver:20.0 Operator : Rev.:A

EN13445:2014 Issue 5:2018+A5 - 10.4 WELDED CIRCULAR FLAT END

E4.1 Flat End 09 Feb. 2021 14:41 ConnID:S1.1

$$=(7.1-0.5-0.71)*\text{MIN}(645,30.79)/30.79=$$

5.8900 mm

Minimum allowable design stress fmin

$$f_{\text{min}} = \text{Min}(f, f_s) = \text{Min}(209.33, 202.67) =$$

202.67 N/mm²

10.4.5 FLAT END WITH RELIEF GROOVE

Minimum required groove radius rdmin

$$rd_{\text{min}} = \text{Max}(0.25 * e_s, 5) = \text{Max}(0.25 * 5.89, 5) =$$

5.0000 mm

»Groove Radius Size rd=5 >= rdmin=5[mm] (10.4.2.4)« » OK«

»Groove Radius Location ern+rd=18 <= en=18[mm] (10.4.2.4)« » OK«

Minimum required thickness at bottom of groove excl.corrosion ermin

$$er_{\text{min}} = \text{Max}(e_s, e_s * (f_s / f))$$

$$= \text{Max}(5.89, 5.89 * (202.67 / 209.33)) =$$

5.8900 mm

Minimum thickness at bottom of groove incl.corr. er

$$er = er_{\text{min}} + c = 5.89 + 0.5 =$$

6.3900 mm

Thickness Check ern=13 >= e=6.39[mm]

49.1%

OK

Factor C1 from fig.10.4.4(es/Di=0.0380) (p/f=0.0642) C1 =0.3939

Factor C2 from fig.10.4.5(es/Di=0.0380) (p/fmin=0.0642) C2 =0.4941

10.4.4 MINIMUM THICKNESS OF FLAT END e

$$e_{\text{min1}} = C1 * Di * \text{Sqr}(P / f) \quad (10.4-12)$$

$$= 0.3939 * 155.1 * \text{Sqr}(13. / 209.33) =$$

15.23 mm

$$e_{\text{min2}} = C2 * Di * \text{Sqr}(P / f_{\text{min}}) \quad (10.4-10)$$

$$= 0.4941 * 155.1 * \text{Sqr}(13. / 202.67) =$$

19.41 mm

NOTE: The design has been based on emin1 only, it is required to perform a simplified assessment of the fatigue life of the flat end to shell junction according to section 17 using etta=

= 3.83 (ref. 10.4.4.4).

Minimum thickness excluding corrosion emin

$$e_{\text{min}} = \text{MAX}(e_{\text{min1}}) = \text{MAX}(15.23) =$$

15.23 mm

Minimum thickness including corrosion e

$$e = e_{\text{min}} + c = 15.23 + 0.5 =$$

15.73 mm

Thickness Check en=18 >= e=15.73[mm]

87.3%

OK

PRESSURE CALCULATIONS

MAXIMUM ALLOWABLE WORKING PRESSURE MAWP :NEW & COLD

$$P_{\text{max}} = f * (e_a / (C1 * Di)) ^ 2$$

$$= 212.5 * (18 / (0.402 * 155.1)) ^ 2 =$$

17.71 MPa

MAXIMUM ALLOWABLE WORKING PRESSURE MAWP :HOT & CORR

$$P_{\text{max}} = f * (e_a / (C1 * Di)) ^ 2$$

$$= 209.33 * (17.5 / (0.4006 * 155.1)) ^ 2 =$$

16.61 MPa

MAX TEST PRESSURE (Uncorroded cond.at ambient temp.)

$$P_{\text{max}} = f * (e_a / (C1 * Di)) ^ 2$$

$$= 328.57 * (18 / (0.4011 * 155.1)) ^ 2 =$$

27.50 MPa

EN13445-5;10.2.3.3 REQUIRED MIN.HYDROSTATIC TEST PRESSURE:Ptmin

NEW AT AMBIENT TEMP. FOR TEST GROUPS 1, 2 and 3

$$Pt_{\text{min}} = 1.25 * Pd * f_{20} / f = 1.25 * 13 * 212.5 / 209.33 =$$

16.50 MPa

$$Pt_{\text{min}} = 1.43 * Pd = 1.43 * 13 =$$

18.59 MPa

Test Pressure Ptmin=18.59 <= Pmax=27.5[MPa]

67.5%

OK

Company Name -

Client :GÜVEN SOGUTMA

Vessel Tag No.:OS.C.130B.35.1

Visual Vessel Design by Hexagon PPM,Ver:20.0 Operator : Rev.:A

EN13445:2014 Issue 5:2018+A5 - 10.4 WELDED CIRCULAR FLAT END

E4.1 Flat End 09 Feb. 2021 14:41 ConnID:S1.1

CALCULATION SUMMARY**10.4.5 FLAT END WITH RELIEF GROOVE**

Minimum thickness at bottom of groove incl.corr. er

er = ermin + c =5.89+0.5=

6.3900 mm**Thickness Check er=13 >= e=6.39[mm]****49.1%****OK****10.4.4 MINIMUM THICKNESS OF FLAT END e**

Minimum thickness including corrosion e

e = emin + c =15.23+0.5=

15.73 mm**Thickness Check en=18 >= e=15.73[mm]****87.3%****OK****PRESSURE CALCULATIONS****MAXIMUM ALLOWABLE WORKING PRESSURE MAWP :NEW & COLD** $P_{max} = f * (ea / (C1 * Di)) ^ 2$

=212.5*(18/(0.402*155.1))^2=

17.71 MPa**MAXIMUM ALLOWABLE WORKING PRESSURE MAWP :HOT & CORR** $P_{max} = f * (ea / (C1 * Di)) ^ 2$

=209.33*(17.5/(0.4006*155.1))^2=

16.61 MPa**MAX TEST PRESSURE (Uncorroded cond.at ambient temp.)** $P_{max} = f * (ea / (C1 * Di)) ^ 2$

=328.57*(18/(0.4011*155.1))^2=

27.50 MPa**EN13445-5;10.2.3.3 REQUIRED MIN.HYDROSTATIC TEST PRESSURE:Ptmin**

NEW AT AMBIENT TEMP. FOR TEST GROUPS 1, 2 and 3

Ptmin = 1.25 * Pd * f20 / f =1.25*13*212.5/209.33=

16.50 MPa

Ptmin = 1.43 * Pd =1.43*13=

18.59 MPa**Test Pressure Ptmin=18.59 <= Pmax=27.5[MPa]****67.5%****OK**

Volume:0.00 m3 Weight:3 kg (SG= 7.85)

Company Name -

Client :GÜVEN SOGUTMA

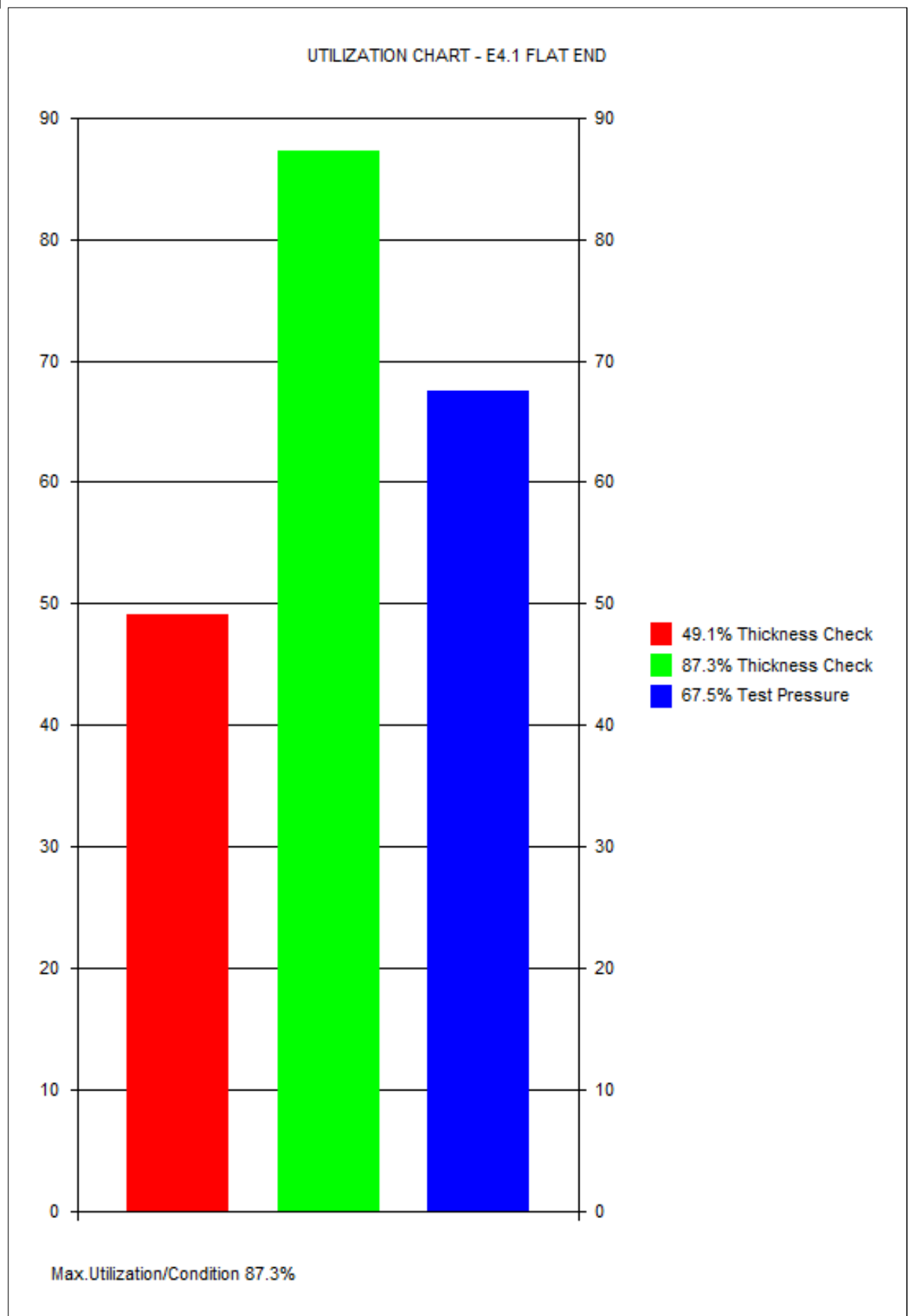
Vessel Tag No.:OS.C.130B.35.1

Visual Vessel Design by Hexagon PPM,Ver:20.0 Operator : Rev.:A

EN13445:2014 Issue 5:2018+A5 - 10.4 WELDED CIRCULAR FLAT END

E4.1 Flat End

09 Feb. 2021 14:41 ConnID:S1.1



Company Name -

Client :GÜVEN SOGUTMA

Vessel Tag No.:OS.C.130B.35.1

Visual Vessel Design by Hexagon PPM,Ver:20.0 Operator : Rev.:A

EN13445:2014 Issue 5:2018+A5 - 10.5 BOLTED CIRCULAR FLAT END

E5.1 Cover Flange 14 Jan. 2021 10:25 ConnID:F.2

INPUT DATA

COMPONENT ATTACHMENT/LOCATION

Attachment: F.2 RT - Flange Shell Flange S1.1
Location: Along z-axis z1= 682

GENERAL DESIGN DATA

PROCESS CARD: General Design Data : Temp= 100°C, P=13.0000 MPa, c=0.5 mm
SPECIFIC DENSITY OF OPERATING LIQUID.....:SG 1.2000
LIQUID HEAD.....:LH 0.00 mm
Check Deflection of Cover to TEMA RCB-9-21(multipass units): NO
Shape of Cover: Circular
Stayed Flat Plate to section 20.2: NO
Circular flat ends with radial reinforcement ribs to section 21: NO

DATA FOR MATING FLANGE (F.2)

TYPE OF BLIND FLANGE: Blind flange with full face gasket
OUTSIDE DIAMETER OF FLANGE.....:A 223.00 mm
BOLT-CIRCLE DIAMETER.....:C 195.00 mm
NUMBER OF BOLTS.....:n 16.00
DIAMETER OF BOLT HOLES IN FLANGE.....:d 16.00 mm
GASKET FACTOR.....:m 0.2500

DATA FOR BLIND FLANGE

NOMINAL THICKNESS OF HEAD/END (uncorroded).....:en 23.00 mm
THICKNESS OF FLANGE(uncorroded).....:e 23.00 mm
EN 10028-2:2017, 1.0473 P355GH plate and strip, HT:N THK<=40mm 100'C
Rm=510 Rp=345 Rpt=314 f=209.33 f20=212.5 ftest=328.57 E=206067(N/mm2) ro=7.85

WELDING REQUIREMENTS TO EN 1708-1:2010

Comment(Optional):
Type of welded connection: Not Applicable

CALCULATION DATA

10.5.3.1 MINIMUM THICKNESS OF FLAT END WITH FULL FACE GASKET e

Minimum thickness excluding corrosion emin
 $e_{min} = 0.41 * C * \text{Sqr}(P / f)$ (10.5-7)
 $= 0.41 * 195 * \text{Sqr}(13 / 209.33) =$ 19.92 mm
Minimum thickness including allowance e
 $e = e_{min} + c = 19.92 + 0.5 =$ 20.42 mm

End Thickness en=23 >= e=20.42[mm]	88.7%	OK
------------------------------------	-------	----

10.5.3.2 MINIMUM THICKNESS OF FLANGED EXTENSION e1

$e_1 = 0.8 * e_{min} (10.5-8) = 0.8 * 19.92 =$ 15.94 mm

Flanged Extension Thk. eb=23 >= e1=15.94[mm]	69.3%	OK
--	-------	----

PRESSURE CALCULATIONS

MAXIMUM ALLOWABLE WORKING PRESSURE MAWP :NEW & COLD

$P_{max} = f * (e_a / (0.41 * C)) ^ 2$
 $= 212.5 * (23 / (0.41 * 195)) ^ 2 =$ 17.59 MPa

MAXIMUM ALLOWABLE WORKING PRESSURE MAWP :HOT & CORR

$P_{max} = f * (e_a / (0.41 * C)) ^ 2$
 $= 209.33 * (22.5 / (0.41 * 195)) ^ 2 =$ 16.58 MPa

Company Name -

Client :GÜVEN SOGUTMA

Vessel Tag No.:OS.C.130B.35.1

Visual Vessel Design by Hexagon PPM,Ver:20.0 Operator : Rev.:A

EN13445:2014 Issue 5:2018+A5 - 10.5 BOLTED CIRCULAR FLAT END

E5.1 Cover Flange 14 Jan. 2021 10:25 ConnID:F.2

MAX TEST PRESSURE (Uncorroded cond.at ambient temp.)

$$P_{max} = f * (ea / (0.41 * C)) ^ 2$$
$$= 328.57 * (23 / (0.41 * 195)) ^ 2 =$$

27.19 MPa

EN13445-5;10.2.3.3 REQUIRED MIN.HYDROSTATIC TEST PRESSURE:Ptmin

NEW AT AMBIENT TEMP. FOR TEST GROUPS 1, 2 and 3

$$Ptmin = 1.25 * Pd * f_{20} / f = 1.25 * 13 * 212.5 / 209.33 =$$

16.50 MPa

$$Ptmin = 1.43 * Pd = 1.43 * 13 =$$

18.59 MPa**Test Pressure Ptmin=18.59 <= Pmax=27.19[MPa]****68.3%****OK**

CALCULATION SUMMARY

10.5.3.1 MINIMUM THICKNESS OF FLAT END WITH FULL FACE GASKET e

Minimum thickness including allowance e

$$e = e_{min} + c = 19.92 + 0.5 =$$

20.42 mm**End Thickness en=23 >= e=20.42[mm]****88.7%****OK**

10.5.3.2 MINIMUM THICKNESS OF FLANGED EXTENSION e1

$$e1 = 0.8 * e_{min} (10.5-8) = 0.8 * 19.92 =$$

15.94 mm**Flanged Extension Thk. eb=23 >= e1=15.94[mm]****69.3%****OK**

PRESSURE CALCULATIONS

MAXIMUM ALLOWABLE WORKING PRESSURE MAWP :NEW & COLD

$$P_{max} = f * (ea / (0.41 * C)) ^ 2$$
$$= 212.5 * (23 / (0.41 * 195)) ^ 2 =$$

17.59 MPa

MAXIMUM ALLOWABLE WORKING PRESSURE MAWP :HOT & CORR

$$P_{max} = f * (ea / (0.41 * C)) ^ 2$$
$$= 209.33 * (22.5 / (0.41 * 195)) ^ 2 =$$

16.58 MPa

MAX TEST PRESSURE (Uncorroded cond.at ambient temp.)

$$P_{max} = f * (ea / (0.41 * C)) ^ 2$$
$$= 328.57 * (23 / (0.41 * 195)) ^ 2 =$$

27.19 MPa

EN13445-5;10.2.3.3 REQUIRED MIN.HYDROSTATIC TEST PRESSURE:Ptmin

NEW AT AMBIENT TEMP. FOR TEST GROUPS 1, 2 and 3

$$Ptmin = 1.25 * Pd * f_{20} / f = 1.25 * 13 * 212.5 / 209.33 =$$

16.50 MPa

$$Ptmin = 1.43 * Pd = 1.43 * 13 =$$

18.59 MPa**Test Pressure Ptmin=18.59 <= Pmax=27.19[MPa]****68.3%****OK**

Volume:0.00 m3 Weight:7 kg (SG= 7.85)

Company Name -

Client :GÜVEN SOGUTMA

Vessel Tag No.:OS.C.130B.35.1

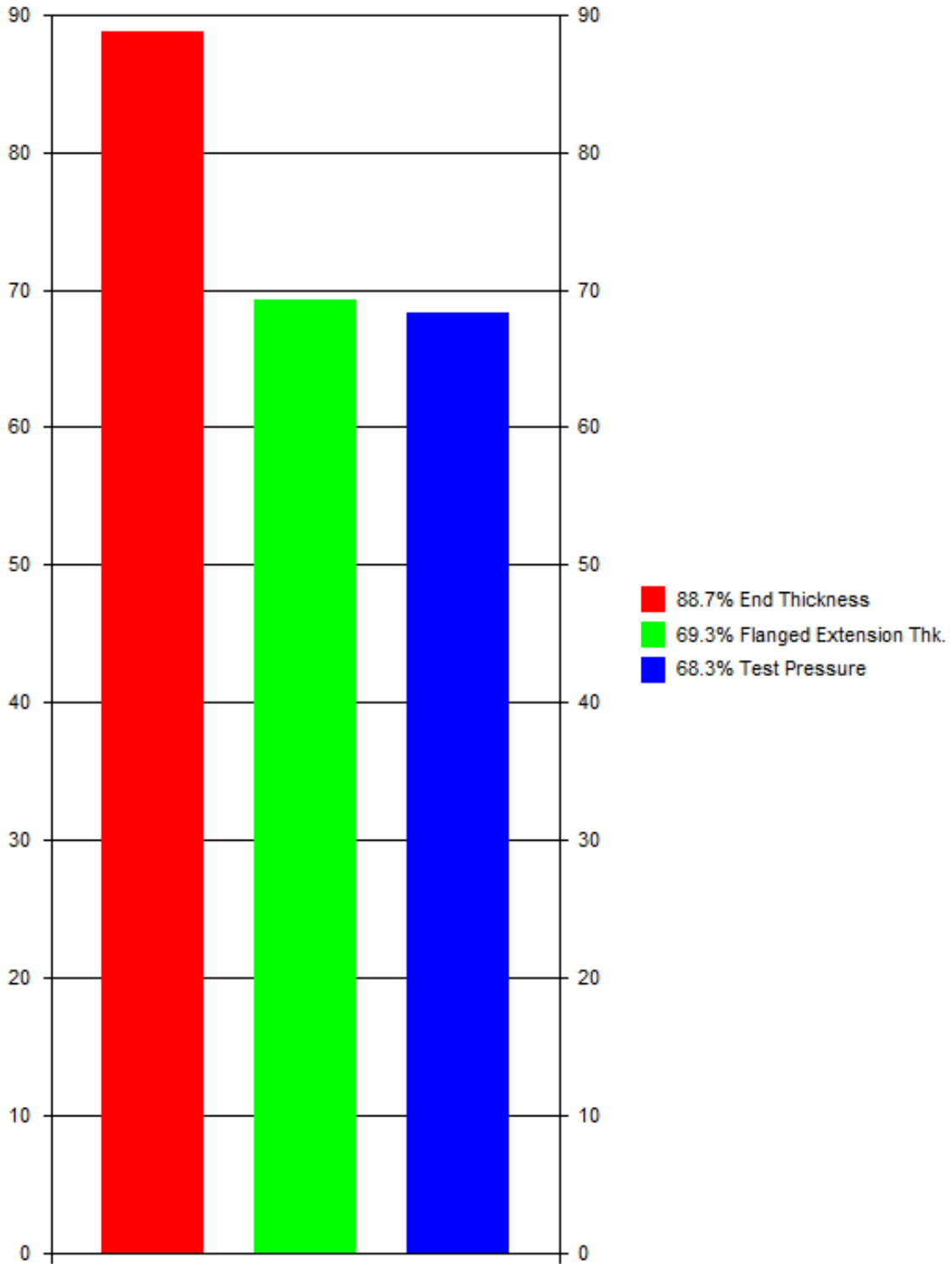
Visual Vessel Design by Hexagon PPM,Ver:20.0 Operator : Rev.:A

EN13445:2014 Issue 5:2018+A5 - 10.5 BOLTED CIRCULAR FLAT END

E5.1 Cover Flange

14 Jan. 2021 10:25 ConnID:F.2

UTILIZATION CHART - E5.1 COVER FLANGE



Max.Utilization/Condition 88.7%

Company Name -

Client :GÜVEN SOGUTMA

Vessel Tag No.:OS.C.130B.35.1

Visual Vessel Design by Hexagon PPM,Ver:20.0 Operator : Rev.:A

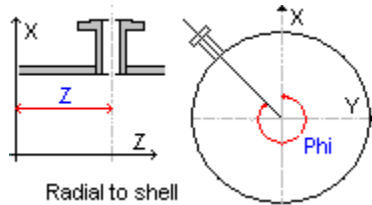
EN13445:2014 Issue 5:2018+A5 - 9.5 ISOLATED OPENINGS IN SHELLS

N.1 Inlet 14 Jan. 2021 10:34 ConnID:S1.1

INPUT DATA

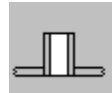
COMPONENT ATTACHMENT/LOCATION

Attachment: S1.1 Cylindrical Shell Shell
Connect this nozzle to the nozzle neck of another nozzle: NO



Orientation & Location of Nozzle: Radial to Shell
z-location of nozzle along axis of attachment.....:z 115.00 mm
Angle of Rotation of nozzle axis projected in the x-y plane:Phi 0.00 Degr.

GENERAL DESIGN DATA



Type of Opening: Nozzle Without Standard ASME or DIN/EN Flange Attachment
PRESSURE LOADING: Design Component for Internal Pressure Only
PROCESS CARD:
General Design Data : Temp= 100°C, P=13.0000 MPa, c=0.5 mm, Pext=0.0000 MPa
SPECIFIC DENSITY OF OPERATING LIQUID.....:SG 1.2000
LIQUID HEAD.....:LH 185.00 mm
Apply a different corrosion allowance to nozzle neck than the shell thickness.: NO
Include Nozzle Load Calculation: NO

SHELL DATA (S1.1)

Shell Type: Cylindrical Shell
OUTSIDE DIAMETER OF SHELL.....:De 168.30 mm
NOMINAL WALL THICKNESS (uncorroded).....:en 7.1000 mm
NEGATIVE TOLERANCE/THINNING ALLOWANCE.....:th 0.7100 mm
EN 10217-3:2002/A1:05, 1.0565 P355NH welded tube, HT:N THK<=20mm 100'C
Rm=490 Rp=355 Rpt=304 fs=202.67 f20=204.17 ftest=338.1 E=206067(N/mm2) ro=7.85

NOZZLE MATERIAL DATA



Delivery Form: Seamless Pipe
EN 10216-2:2013, 1.0425 P265GH seamless tube, HT:N THK<=16mm 100'C
Rm=410 Rp=265 Rpt=226 fb=150.67 f20=170.83 ftest=252.38 E=206067(N/mm2) ro=7.85

NOZZLE DIMENSIONAL DATA

Company Name -

Client :GÜVEN SOGUTMA

Vessel Tag No.:OS.C.130B.35.1

Visual Vessel Design by Hexagon PPM,Ver:20.0 Operator : Rev.:A

EN13445:2014 Issue 5:2018+A5 - 9.5 ISOLATED OPENINGS IN SHELLS

N.1 Inlet 14 Jan. 2021 10:34 ConnID:S1.1



Attachment: Set In Flush Nozzle

Shape of Nozzle/Opening: Circular

Application:

9.4.6.3 NOT a critical fatigue area, and calc.temp.is outside creep range.

OUTSIDE NOZZLE DIAMETER.....:deb 44.50 mm

NOMINAL NOZZLE THICKNESS (uncorroded).....:enb 8.0000 mm

Size of Flange and Nozzle:

Comment (Optional):

NEGATIVE TOLERANCE/THINNING ALLOWANCE.....: 12.50 %

NOZZLE STANDOUT MEASURED FROM VESSEL OD.....:ho 40.00 mm

WELDING DATA

Nozzle/Pad to Shell Welding Area: User Specified Fillet Weld Throat Dimensions

OUTWARD NOZZLE WELD, THROAT DIMENSION.....:mo 5.0000 mm

Nozzle Weld Intersect: Nozzle Does NOT Intersect with a Welded Shell Seam

ANGLE PhiC(OBLIQUE IN TRANSVERSE.CROSS SECT.)Fig.9.5-2:PhiC 0.00 Degr.

ANGLE PhiL(OBLIQUE IN LONG.CROSS SECT.)Fig.9.5-1.....:PhiL 0.00 Degr.

DATA FOR REINFORCEMENT PAD



Type of Pad: No Pad

LIMITS OF REINFORCEMENT

Reduction of Limits of Reinforcement: No Reduction Required

WELDING REQUIREMENTS TO EN 1708-1:2010

Comment(Optional):

Type of welded connection: Not Applicable

CALCULATION DATA

PRELIMINARY CALCULATIONS

Shell Analysis Thickness eas
 $eas = en - c - th = 7.1 - 0.5 - 0.71 = 5.8900$ mm

Nozzle Analysis Thickness eab
 $eab = enb - cn - NegDev = 8 - 0.5 - 1 = 6.5000$ mm

Inside Radius of Curvature
 $ris = De / 2 - eas (9.5-3) = 168.3 / 2 - 5.89 = 78.26$ mm
 $dib = deb - 2 * eab = 44.5 - 2 * 6.5 = 31.50$ mm

Min.Nozzle Thk.Based on Internal Pressure ebp
 $ebp = P * deb / (2 * fb * z + P) = 13. * 44.5 / (2 * 150.67 * 1 + 13.) = 1.8400$ mm

Allowable Stresses
 $fob = Min(fs, fb) (9.5-8) = Min(202.67, 150.67) = 150.67$ N/mm²

GEOMETRIC LIMITATIONS

»Check Max.Diameter of Nozzle $dib / (2 * ris) = 0.2013 \leq 1.00 = 1$ [mm] «» OK«

Min.Nozzle Thk. ebp=1.84 <= eab=6.5 [mm]	28.3%	OK
--	-------	----

9.5.2.4.4 Nozzles normal to the shell, with or without reinforcement pads.

Calculation of Stress Loaded Areas Effective as Reinforcement

Company Name -

Client :GÜVEN SOGUTMA

Vessel Tag No.:OS.C.130B.35.1

Visual Vessel Design by Hexagon PPM,Ver:20.0 Operator : Rev.:A

EN13445:2014 Issue 5:2018+A5 - 9.5 ISOLATED OPENINGS IN SHELLS

N.1 Inlet 14 Jan. 2021 10:34 ConnID:S1.1

Area of Shell Afs

Limit of Reinforcement Along Shell

 $Iso = \text{Sqr}((2 * ris + eas) * eas)$ $= \text{Sqr}((2 * 78.26 + 5.89) * 5.89) =$

30.93 mm

Set In Nozzle

 $Afs = eas * Iso (9.5-79) = 5.89 * 30.93 =$ 182.17 mm²

Area of Nozzle Afb

Limit of Reinforcement Along Nozzle (outside shell)

 $Ibo = \text{MIN}(\text{Sqr}((deb - eb) * eb), ho)$

(9.5-76)

 $= \text{MIN}(\text{Sqr}((44.5 - 6.5) * 6.5), 40) =$

15.72 mm

Set In Nozzle

 $Afb = eb * (Ibo + Ibi + eas) (9.5-78) = 6.5 * (15.72 + 0 + 5.89) =$ 140.44 mm²

Area of Welds Afw

 $Afw = mo^2 = 5^2 =$ 25.00 mm²

Calculation of Pressure Loaded Areas

In the Nozzle Apb

 $Apb = 0.5 * dib * (Ibo + eas) (9.5-84) = 0.5 * 31.5 * (15.72 + 5.89) =$ 340.30 mm²

Cyl.Shell in the Longitudinal Section ApsL

 $ApsL = ris * (Is + a) (9.5-94) = 78.26 * (30.93 + 22.25) =$ 4161.78 mm²

Cyl.Shell in the Transverse Cross Section ApsT

 $ApsT = 0.5 * ris^2 * (Is + a) / (0.5 * eas + ris)$

(9.5-105)

 $= 0.5 * 78.26^2 * (30.93 + 22.54) / (0.5 * 5.89 + 78.26) =$ 2016.29 mm² $Aps = \text{MAX}(ApsL, ApsT) = \text{MAX}(4161.78, 2016.29) =$ 4161.78 mm²

9.5.2 Reinforcement Rules

Pressure Area Required pA(req.)

 $pAReqL = P * (ApsL + Apb) (9.5-7) = 13 * (4161.78 + 340.3) =$ 58.54 kN $pAReqT = P * (ApsT + Apb + 0.5 * Apphi)$

(9.5-7)

 $= 13 * (2016.29 + 340.3 + 0.5 * 0) =$ 30.64 kN $pAReq = \text{MAX}(pAReqL, pAReqT) = \text{MAX}(58536.9, 30640.87) =$ 58.54 kN

Pressure Area Available pA(aval.)

 $pAAval = (Afs + Afw) * (fs - 0.5 * P) + Afp * (fop - 0.5 * P) + Afb * (fob - 0.5 * P) (9.5-7)$ $= (182.17 + 25) * (202.67 - 0.5 * 13.) + 0 * (0 - 0.5 * 13.) + 140.44 * (150.67 - 0.5 * 13.)$ $= 60.89 \text{ kN}$ **Nozzle Reinforcement pAAval=60.89 >= pAReq=58.54[kN]****96.1%****OK**

Maximum Allowable Pressure Pmax

 $Pmax = (Afs + Afw) * fs + Afb * fob / ((ApsL + Apb) + 0.5 * (Afs + Afw + Afb + Afp)) (9.5-10)$ $= (182.17 + 25) * 202.67 + 140.44 * 150.67 / ((4161.78 + 340.3) + 0.5 * (182.17 + 25 + 140.44 + 0))$ $=$ 13.50 MPa

Max.Allowable Test Pressure Pmax

 $Ptmax = ==$ 24.19 MPa

Weight of Nozzle: .3628kg

CALCULATION SUMMARY

Min.Nozzle Thk. ebp=1.84 <= eab=6.5[mm]**28.3%****OK**

Company Name -

Client :GÜVEN SOGUTMA

Vessel Tag No.:OS.C.130B.35.1

Visual Vessel Design by Hexagon PPM,Ver:20.0 Operator : Rev.:A

EN13445:2014 Issue 5:2018+A5 - 9.5 ISOLATED OPENINGS IN SHELLS

N.1 Inlet 14 Jan. 2021 10:34 ConnID:S1.1

9.5.2.4 Nozzles normal to the shell, with or without reinforcement pads.

Limit of Reinforcement Along Shell

$$Iso = \text{Sqr}((2 * ris + eas) * eas)$$

$$= \text{Sqr}((2 * 78.26 + 5.89) * 5.89) =$$

30.93 mm

Limit of Reinforcement Along Nozzle (outside shell)

$$Ibo = \text{MIN}(\text{Sqr}((deb - eb) * eb), ho)$$

(9.5-76)

$$= \text{MIN}(\text{Sqr}((44.5 - 6.5) * 6.5), 40) =$$

15.72 mm

Pressure Area Required pA(req.)

$$pAReqL = P * (ApsL + Apb) \quad (9.5-7) = 13. * (4161.78 + 340.3) =$$

58.54 kN

$$pAReqT = P * (ApsT + Apb + 0.5 * Apphi)$$

(9.5-7)

$$= 13. * (2016.29 + 340.3 + 0.5 * 0) =$$

30.64 kN

$$pAReq = \text{MAX}(pAReqL, pAReqT) = \text{MAX}(58536.9, 30640.87) =$$

58.54 kN

Pressure Area Available pA(aval.)

$$pAAval = (Afs + Afw) * (fs - 0.5 * P) + Afp * (fop - 0.5 * P) + Afb * (fob - 0.5 * P) \quad (9.5-7)$$

$$= (182.17 + 25) * (202.67 - 0.5 * 13.) + 0 * (0 - 0.5 * 13.) + 140.44 * (150.67 - 0.5 * 13.)$$

$$= 60.89 \text{ kN}$$

Nozzle Reinforcement pAAval=60.89 >= pAReq=58.54[kN]**96.1%****OK****Maximum Allowable Pressure Pmax**

$$Pmax = (Afs + Afw) * fs + Afb * fob / ((ApsL + Apb) + 0.5 * (Afs + Afw + Afb + Afp)) \quad (9.5-10)$$

$$= (182.17 + 25) * 202.67 + 140.44 * 150.67 / ((4161.78 + 340.3) + 0.5 * (182.17 + 25 + 140.44 + 0))$$

$$)=$$

= 13.50 MPa

Volume:0.00 m3 Weight:0.4 kg (SG= 7.85)

Company Name -

Client :GÜVEN SOGUTMA

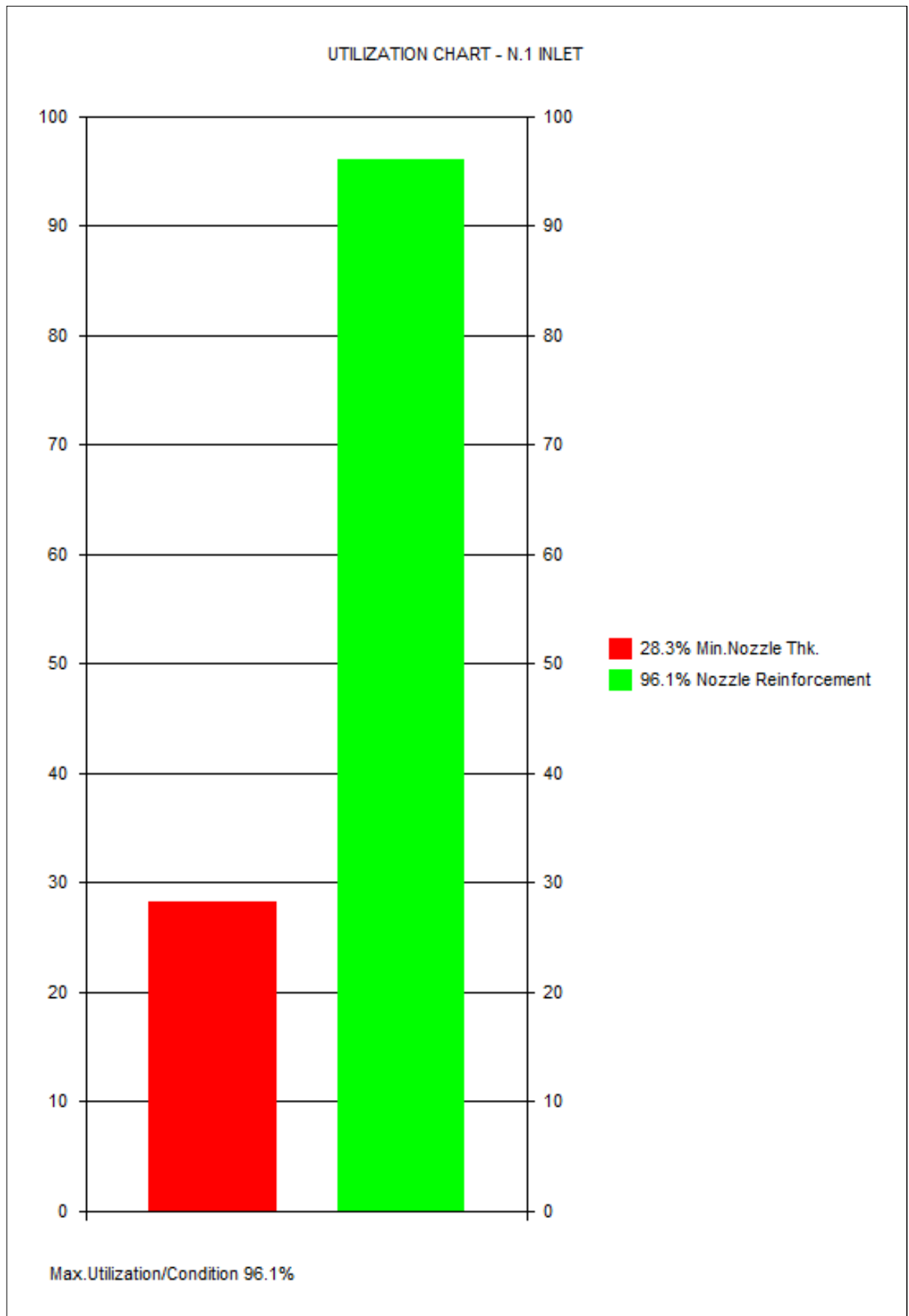
Vessel Tag No.:OS.C.130B.35.1

Visual Vessel Design by Hexagon PPM,Ver:20.0 Operator : Rev.:A

EN13445:2014 Issue 5:2018+A5 - 9.5 ISOLATED OPENINGS IN SHELLS

N.1 Inlet

14 Jan. 2021 10:34 ConnID:S1.1



Company Name -

Client :GÜVEN SOGUTMA

Vessel Tag No.:OS.C.130B.35.1

Visual Vessel Design by Hexagon PPM,Ver:20.0 Operator : Rev.:A

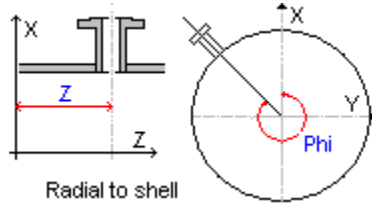
EN13445:2014 Issue 5:2018+A5 - 9.5 ISOLATED OPENINGS IN SHELLS

N.2 Outlet 14 Jan. 2021 14:03 ConnID:S1.1

INPUT DATA

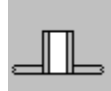
COMPONENT ATTACHMENT/LOCATION

Attachment: S1.1 Cylindrical Shell Shell
Connect this nozzle to the nozzle neck of another nozzle: NO



Orientation & Location of Nozzle: Radial to Shell
z-location of nozzle along axis of attachment.....:z 575.00 mm
Angle of Rotation of nozzle axis projected in the x-y plane:Phi 0.00 Degr.

GENERAL DESIGN DATA



Type of Opening: Nozzle Without Standard ASME or DIN/EN Flange Attachment
PRESSURE LOADING: Design Component for Internal Pressure Only
PROCESS CARD:
General Design Data : Temp= 100°C, P=13.0000 MPa, c=0.5 mm, Pext=0.0000 MPa
SPECIFIC DENSITY OF OPERATING LIQUID.....:SG 1.2000
LIQUID HEAD.....:LH 0.00 mm
Apply a different corrosion allowance to nozzle neck than the shell thickness.: NO
Include Nozzle Load Calculation: NO

SHELL DATA (S1.1)

Shell Type: Cylindrical Shell
OUTSIDE DIAMETER OF SHELL.....:De 168.30 mm
NOMINAL WALL THICKNESS (uncorroded).....:en 7.1000 mm
NEGATIVE TOLERANCE/THINNING ALLOWANCE.....:th 0.7100 mm
EN 10217-3:2002/A1:05, 1.0565 P355NH welded tube, HT:N THK<=20mm 100'C
Rm=490 Rp=355 Rpt=304 fs=202.67 f20=204.17 ftest=338.1 E=206067(N/mm2) ro=7.85

NOZZLE MATERIAL DATA



Delivery Form: Seamless Pipe
EN 10216-2:2013, 1.0425 P265GH seamless tube, HT:N THK<=16mm 100'C
Rm=410 Rp=265 Rpt=226 fb=150.67 f20=170.83 ftest=252.38 E=206067(N/mm2) ro=7.85

NOZZLE DIMENSIONAL DATA

Company Name -

Client :GÜVEN SOGUTMA

Vessel Tag No.:OS.C.130B.35.1

Visual Vessel Design by Hexagon PPM,Ver:20.0 Operator : Rev.:A

EN13445:2014 Issue 5:2018+A5 - 9.5 ISOLATED OPENINGS IN SHELLS

N.2 Outlet 14 Jan. 2021 14:03 ConnID:S1.1



Attachment: Set In Flush Nozzle

Shape of Nozzle/Opening: Circular

Application:

9.4.6.3 NOT a critical fatigue area, and calc.temp.is outside creep range.

OUTSIDE NOZZLE DIAMETER.....:deb 44.50 mm

NOMINAL NOZZLE THICKNESS (uncorroded).....:enb 8.0000 mm

Size of Flange and Nozzle:

Comment (Optional):

NEGATIVE TOLERANCE/THINNING ALLOWANCE.....: 12.50 %

NOZZLE STANDOUT MEASURED FROM VESSEL OD.....:ho 40.00 mm

WELDING DATA

Nozzle/Pad to Shell Welding Area: User Specified Fillet Weld Throat Dimensions

OUTWARD NOZZLE WELD, THROAT DIMENSION.....:mo 5.0000 mm

Nozzle Weld Intersect: Nozzle Does NOT Intersect with a Welded Shell Seam

ANGLE PhiC(OBLIQUE IN TRANSVERSE.CROSS SECT.)Fig.9.5-2:PhiC 0.00 Degr.

ANGLE PhiL(OBLIQUE IN LONG.CROSS SECT.)Fig.9.5-1.....:PhiL 0.00 Degr.

DATA FOR REINFORCEMENT PAD



Type of Pad: No Pad

LIMITS OF REINFORCEMENT

Reduction of Limits of Reinforcement: No Reduction Required

WELDING REQUIREMENTS TO EN 1708-1:2010

Comment(Optional):

Type of welded connection: Not Applicable

CALCULATION DATA

PRELIMINARY CALCULATIONS

Shell Analysis Thickness eas
 $eas = en - c - th = 7.1 - 0.5 - 0.71 = 5.8900$ mm

Nozzle Analysis Thickness eab
 $eab = enb - cn - NegDev = 8 - 0.5 - 1 = 6.5000$ mm

Inside Radius of Curvature
 $ris = De / 2 - eas (9.5-3) = 168.3 / 2 - 5.89 = 78.26$ mm
 $dib = deb - 2 * eab = 44.5 - 2 * 6.5 = 31.50$ mm

Min.Nozzle Thk.Based on Internal Pressure ebp
 $ebp = P * deb / (2 * fb * z + P) = 13 * 44.5 / (2 * 150.67 * 1 + 13) = 1.8400$ mm

Allowable Stresses
 $fob = Min(fs, fb) (9.5-8) = Min(202.67, 150.67) = 150.67$ N/mm²

GEOMETRIC LIMITATIONS

»Check Max.Diameter of Nozzle $dib / (2 * ris) = 0.2013 \leq 1.00 = 1$ [mm] «» OK«

Min.Nozzle Thk. ebp=1.84 <= eab=6.5 [mm]	28.3%	OK
--	-------	----

9.5.2.4.4 Nozzles normal to the shell, with or without reinforcement pads.

Calculation of Stress Loaded Areas Effective as Reinforcement

Company Name -

Client :GÜVEN SOGUTMA

Vessel Tag No.:OS.C.130B.35.1

Visual Vessel Design by Hexagon PPM,Ver:20.0 Operator : Rev.:A

EN13445:2014 Issue 5:2018+A5 - 9.5 ISOLATED OPENINGS IN SHELLS

N.2 Outlet

14 Jan. 2021 14:03 ConnID:S1.1

Area of Shell Afs

Limit of Reinforcement Along Shell

 $Iso = \text{Sqr}((2 * ris + eas) * eas)$ $= \text{Sqr}((2*78.26+5.89)*5.89)=$

30.93 mm

Set In Nozzle

 $Afs = eas * Iso (9.5-79) = 5.89*30.93=$ 182.17 mm²

Area of Nozzle Afb

Limit of Reinforcement Along Nozzle (outside shell)

 $Ibo = \text{MIN}(\text{Sqr}((deb - eb) * eb), ho)$

(9.5-76)

 $= \text{MIN}(\text{Sqr}((44.5-6.5)*6.5), 40)=$

15.72 mm

Set In Nozzle

 $Afb = eb * (Ibo + Ibi + eas) (9.5-78) = 6.5*(15.72+0+5.89)=$ 140.44 mm²

Area of Welds Afw

 $Afw = mo^2 = 5^2=$ 25.00 mm²

Calculation of Pressure Loaded Areas

In the Nozzle Apb

 $Apb = 0.5 * dib * (Ibo + eas) (9.5-84) = 0.5*31.5*(15.72+5.89)=$ 340.30 mm²

Cyl.Shell in the Longitudinal Section ApsL

 $ApsL = ris * (Is + a) (9.5-94) = 78.26*(30.93+22.25)=$ 4161.78 mm²

Cyl.Shell in the Transverse Cross Section ApsT

 $ApsT = 0.5 * ris^2 * (Is + a) / (0.5 * eas + ris)$

(9.5-105)

 $= 0.5*78.26^2*(30.93+22.54)/(0.5*5.89+78.26)=$ 2016.29 mm² $Aps = \text{MAX}(ApsL, ApsT) = \text{MAX}(4161.78, 2016.29)=$ 4161.78 mm²

9.5.2 Reinforcement Rules

Pressure Area Required pA(req.)

 $pAReqL = P * (ApsL + Apb) (9.5-7) = 13*(4161.78+340.3)=$ 58.53 kN $pAReqT = P * (ApsT + Apb + 0.5 * Apphi)$

(9.5-7)

 $= 13*(2016.29+340.3+0.5*0)=$ 30.64 kN $pAReq = \text{MAX}(pAReqL, pAReqT) = \text{MAX}(58527., 30635.69)=$ 58.53 kN

Pressure Area Available pA(aval.)

 $pAAval = (Afs+Afw)*(fs-0.5*P)+Afp*(fop-0.5*P)+Afb*(fob-0.5*P)$

(9.5-7)

 $= (182.17+25)*(202.67-0.5*13)+0*(0-0.5*13)+140.44*(150.67-0.5*13)=$ 60.89 kN**Nozzle Reinforcement pAAval=60.89 >= pAReq=58.53[kN]****96.1%****OK**

Maximum Allowable Pressure Pmax

 $Pmax = (Afs+Afw)*fs+Afb*fob / ((ApsL+Apb)+0.5*(Afs+Afw+Afb+Afp))$

(9.5-10)

 $= (182.17+25)*202.67+140.44*150.67 / ((4161.78+340.3)+0.5*(182.17+25+140.44+0))$ $=$ 13.50 MPa

Max.Allowable Test Pressure Ptmax

 $Ptmax = ==$ 24.19 MPa

Weight of Nozzle: .3628kg

CALCULATION SUMMARY

Min.Nozzle Thk. ebp=1.84 <= eab=6.5[mm]**28.3%****OK**

Company Name -

Client :GÜVEN SOGUTMA

Vessel Tag No.:OS.C.130B.35.1

Visual Vessel Design by Hexagon PPM,Ver:20.0 Operator : Rev.:A

EN13445:2014 Issue 5:2018+A5 - 9.5 ISOLATED OPENINGS IN SHELLS

N.2 Outlet 14 Jan. 2021 14:03 ConnID:S1.1

9.5.2.4 Nozzles normal to the shell, with or without reinforcement pads.

Limit of Reinforcement Along Shell

$$Iso = \text{Sqr}((2 * ris + eas) * eas)$$

$$= \text{Sqr}((2 * 78.26 + 5.89) * 5.89) =$$

30.93 mm

Limit of Reinforcement Along Nozzle (outside shell)

$$Ibo = \text{MIN}(\text{Sqr}((deb - eb) * eb), ho)$$

(9.5-76)

$$= \text{MIN}(\text{Sqr}((44.5 - 6.5) * 6.5), 40) =$$

15.72 mm

Pressure Area Required pA(req.)

$$pAreqL = P * (ApsL + Apb) \quad (9.5-7) = 13 * (4161.78 + 340.3) =$$

58.53 kN

$$pAreqT = P * (ApsT + Apb + 0.5 * Apphi)$$

(9.5-7)

$$= 13 * (2016.29 + 340.3 + 0.5 * 0) =$$

30.64 kN

$$pAreq = \text{MAX}(pAreqL, pAreqT) = \text{MAX}(58527., 30635.69) =$$

58.53 kN

Pressure Area Available pA(aval.)

$$pAAval = (Afs + Afw) * (fs - 0.5 * P) + Afp * (fop - 0.5 * P) + Afb * (fob - 0.5 * P) \quad (9.5-7)$$

$$= (182.17 + 25) * (202.67 - 0.5 * 13) + 0 * (0 - 0.5 * 13) + 140.44 * (150.67 - 0.5 * 13) =$$

60.89 kN

Nozzle Reinforcement pAAval=60.89 >= pAreq=58.53[kN]**96.1%****OK****Maximum Allowable Pressure Pmax**

$$Pmax = (Afs + Afw) * fs + Afb * fob / ((ApsL + Apb) + 0.5 * (Afs + Afw + Afb + Afp)) \quad (9.5-10)$$

$$= (182.17 + 25) * 202.67 + 140.44 * 150.67 / ((4161.78 + 340.3) + 0.5 * (182.17 + 25 + 140.44 + 0))$$

)=

= 13.50 MPa

Volume:0.00 m3 Weight:0.4 kg (SG= 7.85)

Company Name -

Client :GÜVEN SOGUTMA

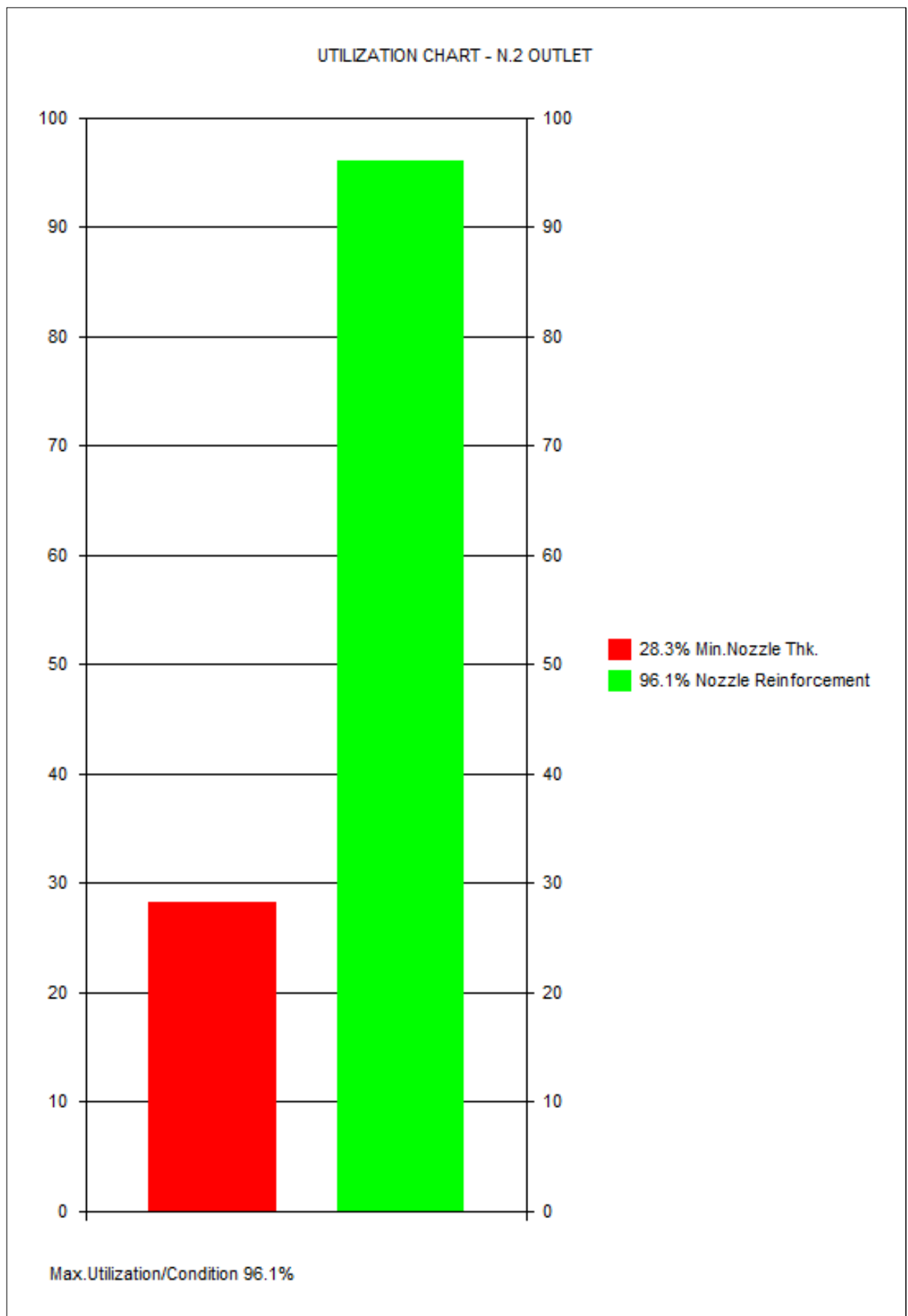
Vessel Tag No.:OS.C.130B.35.1

Visual Vessel Design by Hexagon PPM,Ver:20.0 Operator : Rev.:A

EN13445:2014 Issue 5:2018+A5 - 9.5 ISOLATED OPENINGS IN SHELLS

N.2 Outlet

14 Jan. 2021 14:03 ConnID:S1.1



Company Name -

Client :GÜVEN SOGUTMA

Vessel Tag No.:OS.C.130B.35.1

Visual Vessel Design by Hexagon PPM,Ver:20.0 Operator : Rev.:A

EN13445:2014 Issue 5:2018+A5 - 9.5 ISOLATED OPENINGS IN SHELLS

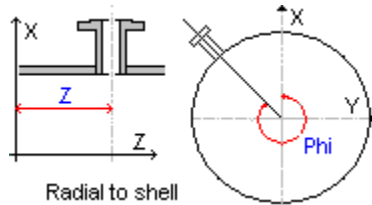
N.3 Safety Valve 14 Jan. 2021 10:38 ConnID:S1.1

INPUT DATA

COMPONENT ATTACHMENT/LOCATION

Attachment: S1.1 Cylindrical Shell Shell

Connect this nozzle to the nozzle neck of another nozzle: NO

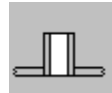


Orientation & Location of Nozzle: Radial to Shell

z-location of nozzle along axis of attachment.....:z 105.00 mm

Angle of Rotation of nozzle axis projected in the x-y plane:Phi 180.00 Degr.

GENERAL DESIGN DATA



Type of Opening: Nozzle Without Standard ASME or DIN/EN Flange Attachment
PRESSURE LOADING: Design Component for Internal Pressure Only

PROCESS CARD:

General Design Data : Temp= 100°C, P=13.0000 MPa, c=0.5 mm, Pext=0.0000 MPa

SPECIFIC DENSITY OF OPERATING LIQUID.....:SG 1.2000

LIQUID HEAD.....:LH 195.00 mm

Apply a different corrosion allowance to nozzle neck than the shell thickness.: NO

Include Nozzle Load Calculation: NO

SHELL DATA (S1.1)

Shell Type: Cylindrical Shell

OUTSIDE DIAMETER OF SHELL.....:De 168.30 mm

NOMINAL WALL THICKNESS (uncorroded).....:en 7.1000 mm

NEGATIVE TOLERANCE/THINNING ALLOWANCE.....:th 0.7100 mm

EN 10217-3:2002/A1:05, 1.0565 P355NH welded tube, HT:N THK<=20mm 100'C

Rm=490 Rp=355 Rpt=304 fs=202.67 f20=204.17 ftest=338.1 E=206067(N/mm2) ro=7.85

NOZZLE MATERIAL DATA



Delivery Form: Seamless Pipe

EN 10216-2:2013, 1.0425 P265GH seamless tube, HT:N THK<=16mm 100'C

Rm=410 Rp=265 Rpt=226 fb=150.67 f20=170.83 ftest=252.38 E=206067(N/mm2) ro=7.85

NOZZLE DIMENSIONAL DATA

Company Name -

Client :GÜVEN SOGUTMA

Vessel Tag No.:OS.C.130B.35.1

Visual Vessel Design by Hexagon PPM,Ver:20.0 Operator : Rev.:A

EN13445:2014 Issue 5:2018+A5 - 9.5 ISOLATED OPENINGS IN SHELLS

N.3 Safety Valve 14 Jan. 2021 10:38 ConnID:S1.1



Attachment: Set In Flush Nozzle

Shape of Nozzle/Opening: Circular

Application:

9.4.6.3 NOT a critical fatigue area, and calc.temp.is outside creep range.

OUTSIDE NOZZLE DIAMETER.....:deb 30.00 mm

NOMINAL NOZZLE THICKNESS (uncorroded).....:enb 7.0000 mm

Size of Flange and Nozzle:

Comment (Optional):

NEGATIVE TOLERANCE/THINNING ALLOWANCE.....: 12.50 %

NOZZLE STANDOUT MEASURED FROM VESSEL OD.....:ho 35.00 mm

WELDING DATA

Nozzle/Pad to Shell Welding Area: User Specified Fillet Weld Throat Dimensions

OUTWARD NOZZLE WELD, THROAT DIMENSION.....:mo 3.5000 mm

Nozzle Weld Intersect: Nozzle Does NOT Intersect with a Welded Shell Seam

ANGLE PhiC(OBLIQUE IN TRANSVERSE.CROSS SECT.)Fig.9.5-2:PhiC 0.00 Degr.

ANGLE PhiL(OBLIQUE IN LONG.CROSS SECT.)Fig.9.5-1.....:PhiL 0.00 Degr.

DATA FOR REINFORCEMENT PAD



Type of Pad: No Pad

LIMITS OF REINFORCEMENT

Reduction of Limits of Reinforcement: No Reduction Required

WELDING REQUIREMENTS TO EN 1708-1:2010

Comment(Optional):

Type of welded connection: Not Applicable

CALCULATION DATA

PRELIMINARY CALCULATIONS

Shell Analysis Thickness eas
eas = en - c - th =7.1-0.5-0.71= 5.8900 mm

Nozzle Analysis Thickness eab
eab = enb - cn - NegDev =7-0.5-0.875= 5.6250 mm

Inside Radius of Curvature
ris = De / 2 - eas (9.5-3) =168.3/2-5.89= 78.26 mm
dib = deb - 2 * eab =30-2*5.625= 18.75 mm

Min.Nozzle Thk.Based on Internal Pressure ebp
ebp = P * deb / (2 * fb * z + P)
=13.*30/(2*150.67*1+13.)= 1.2400 mm

Allowable Stresses
fob = Min(fs, fb) (9.5-8) =Min(202.67,150.67)= 150.67 N/mm2

GEOMETRIC LIMITATIONS

»Check Max.Diameter of Nozzle dib/(2*ris)=0.1198 <= 1.00=1[mm] «» OK«

Min.Nozzle Thk. ebp=1.24 <= eab=5.625[mm]

22.0%

OK

9.5.2.4.4 Nozzles normal to the shell, with or without reinforcement pads.

Calculation of Stress Loaded Areas Effective as Reinforcement

Company Name -

Client :GÜVEN SOGUTMA

Vessel Tag No.:OS.C.130B.35.1

Visual Vessel Design by Hexagon PPM,Ver:20.0 Operator : Rev.:A

EN13445:2014 Issue 5:2018+A5 - 9.5 ISOLATED OPENINGS IN SHELLS

N.3 Safety Valve 14 Jan. 2021 10:38 ConnID:S1.1

Area of Shell Afs

Limit of Reinforcement Along Shell

 $Iso = \text{Sqr}((2 * ris + eas) * eas)$ $= \text{Sqr}((2 * 78.26 + 5.89) * 5.89) =$

30.93 mm

Set In Nozzle

 $Afs = eas * Iso (9.5-79) = 5.89 * 30.93 =$ 182.17 mm²

Area of Nozzle Afb

Limit of Reinforcement Along Nozzle (outside shell)

 $Ibo = \text{MIN}(\text{Sqr}((deb - eb) * eb), ho)$

(9.5-76)

 $= \text{MIN}(\text{Sqr}((30 - 5.625) * 5.625), 35) =$

11.71 mm

Set In Nozzle

 $Afb = eb * (Ibo + Ibi + eas) (9.5-78) = 5.625 * (11.71 + 0 + 5.89) =$ 99.00 mm²

Area of Welds Afw

 $Afw = mo^2 = 3.5^2 =$ 12.25 mm²

Calculation of Pressure Loaded Areas

In the Nozzle Apb

 $Apb = 0.5 * dib * (Ibo + eas) (9.5-84) = 0.5 * 18.75 * (11.71 + 5.89) =$ 164.99 mm²

Cyl.Shell in the Longitudinal Section ApsL

 $ApsL = ris * (Is + a) (9.5-94) = 78.26 * (30.93 + 15) =$ 3594.39 mm²

Cyl.Shell in the Transverse Cross Section ApsT

 $ApsT = 0.5 * ris^2 * (Is + a) / (0.5 * eas + ris)$

(9.5-105)

 $= 0.5 * 78.26^2 * (30.93 + 15.09) / (0.5 * 5.89 + 78.26) =$ 1735.29 mm² $Aps = \text{MAX}(ApsL, ApsT) = \text{MAX}(3594.39, 1735.29) =$ 3594.39 mm²

9.5.2 Reinforcement Rules

Pressure Area Required pA(req.)

 $pAReqL = P * (ApsL + Apb) (9.5-7) = 13. * (3594.39 + 164.99) =$ 48.88 kN $pAReqT = P * (ApsT + Apb + 0.5 * Apphi)$

(9.5-7)

 $= 13. * (1735.29 + 164.99 + 0.5 * 0) =$ 24.71 kN $pAReq = \text{MAX}(pAReqL, pAReqT) = \text{MAX}(48880.68, 24708.02) =$ 48.88 kN

Pressure Area Available pA(aval.)

 $pAAval = (Afs + Afw) * (fs - 0.5 * P) + Afp * (fop - 0.5 * P) + Afb * (fob - 0.5 * P)$

(9.5-7)

 $= (182.17 + 12.25) * (202.67 - 0.5 * 13.) + 0 * (0 - 0.5 * 13.) + 99. * (150.67 - 0.5 * 13.)$ $= 52.41 \text{ kN}$ **Nozzle Reinforcement pAAval=52.41 >= pAReq=48.88[kN]****93.2%****OK**

Maximum Allowable Pressure Pmax

 $Pmax = (Afs + Afw) * fs + Afb * fob / ((ApsL + Apb) + 0.5 * (Afs + Afw + Afb + Afp))$

(9.5-10)

 $= (182.17 + 12.25) * 202.67 + 99. * 150.67 / ((3594.39 + 164.99) + 0.5 * (182.17 + 12.25 + 99. + 0$ 13.91 MPa

Max.Allowable Test Pressure Pmax

 $Ptmax = ==$ 25.05 MPa

Weight of Nozzle: .173kg

CALCULATION SUMMARY

Min.Nozzle Thk. ebp=1.24 <= eab=5.625[mm]**22.0%****OK**

Company Name -

Client :GÜVEN SOGUTMA

Vessel Tag No.:OS.C.130B.35.1

Visual Vessel Design by Hexagon PPM,Ver:20.0 Operator : Rev.:A

EN13445:2014 Issue 5:2018+A5 - 9.5 ISOLATED OPENINGS IN SHELLS

N.3 Safety Valve 14 Jan. 2021 10:38 ConnID:S1.1

9.5.2.4 Nozzles normal to the shell, with or without reinforcement pads.

Limit of Reinforcement Along Shell

$$Iso = \text{Sqr}((2 * ris + eas) * eas)$$

$$= \text{Sqr}((2 * 78.26 + 5.89) * 5.89) =$$

30.93 mm

Limit of Reinforcement Along Nozzle (outside shell)

$$Ibo = \text{MIN}(\text{Sqr}((deb - eb) * eb), ho)$$

(9.5-76)

$$= \text{MIN}(\text{Sqr}((30 - 5.625) * 5.625), 35) =$$

11.71 mm

Pressure Area Required pA(req.)

$$pAReqL = P * (ApsL + Apb) \quad (9.5-7) = 13. * (3594.39 + 164.99) =$$

48.88 kN

$$pAReqT = P * (ApsT + Apb + 0.5 * Apphi)$$

(9.5-7)

$$= 13. * (1735.29 + 164.99 + 0.5 * 0) =$$

24.71 kN

$$pAReq = \text{MAX}(pAReqL, pAReqT) = \text{MAX}(48880.68, 24708.02) =$$

48.88 kN

Pressure Area Available pA(aval.)

$$pAAval = (Afs + Afw) * fs + Afb * fob + Afp * (fop - 0.5 * P) + Afb * (fob - 0.5 * P) \quad (9.5-7)$$

$$= (182.17 + 12.25) * (202.67 - 0.5 * 13.) + 0 * (0 - 0.5 * 13.) + 99. * (150.67 - 0.5 * 13.)$$

$$= 52.41 \text{ kN}$$

Nozzle Reinforcement pAAval=52.41 >= pAReq=48.88[kN]**93.2%****OK****Maximum Allowable Pressure Pmax**

$$Pmax = (Afs + Afw) * fs + Afb * fob / ((ApsL + Apb) + 0.5 * (Afs + Afw + Afb + Afp)) \quad (9.5-10)$$

$$= (182.17 + 12.25) * 202.67 + 99. * 150.67 / ((3594.39 + 164.99) + 0.5 * (182.17 + 12.25 + 99. + 0)) =$$

13.91 MPa

Volume:0.00 m3 Weight:0.2 kg (SG= 7.85)

Company Name -

Client :GÜVEN SOGUTMA

Vessel Tag No.:OS.C.130B.35.1

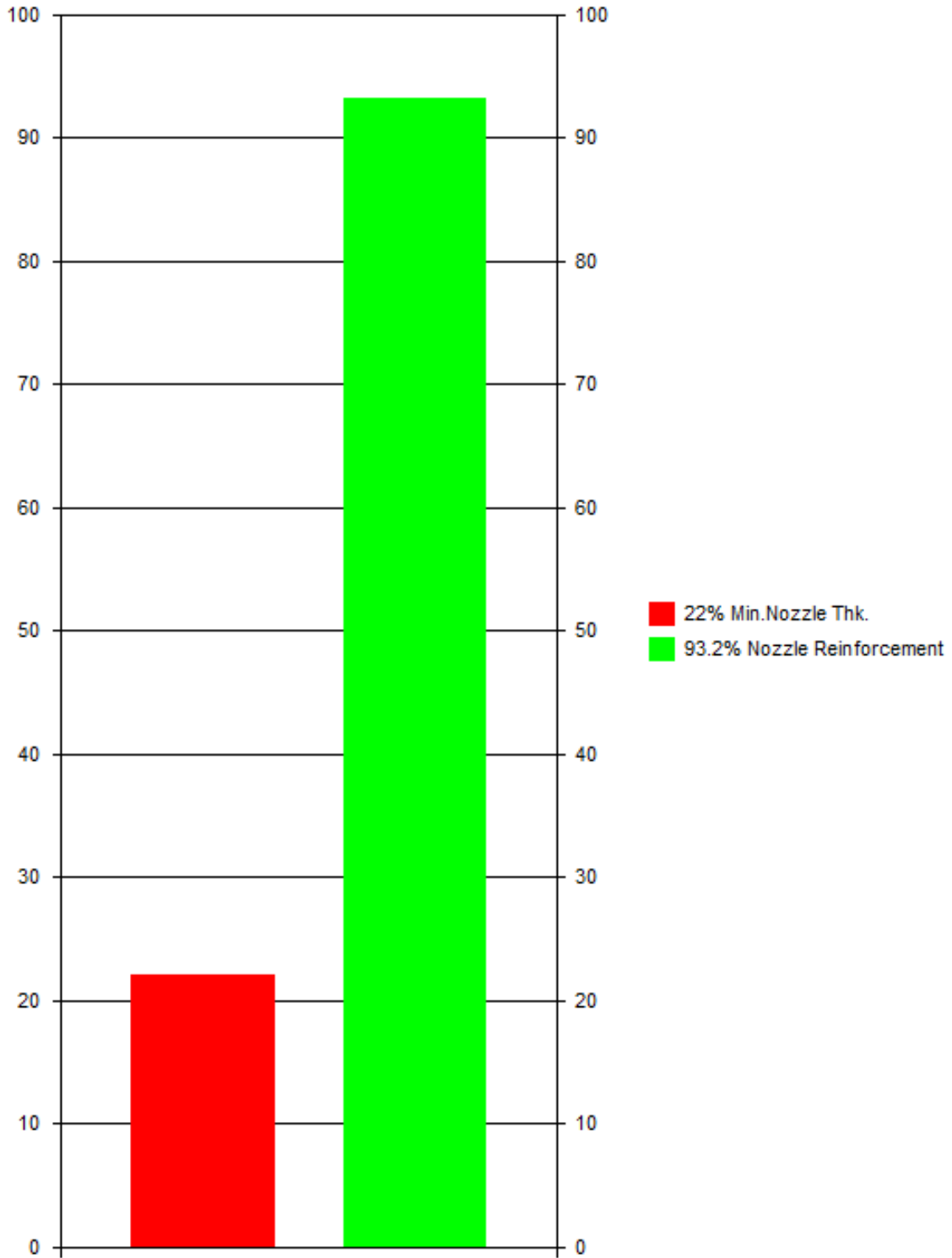
Visual Vessel Design by Hexagon PPM,Ver:20.0 Operator : Rev.:A

EN13445:2014 Issue 5:2018+A5 - 9.5 ISOLATED OPENINGS IN SHELLS

N.3 Safety Valve

14 Jan. 2021 10:38 ConnID:S1.1

UTILIZATION CHART - N.3 SAFETY VALVE



Max.Utilization/Condition 93.2%

Company Name -

Client :GÜVEN SOGUTMA

Vessel Tag No.:OS.C.130B.35.1

Visual Vessel Design by Hexagon PPM,Ver:20.0 Operator : Rev.:A

EN13445:2014 Issue 5:2018+A5 - 9.5 ISOLATED OPENINGS IN SHELLS

N.4 Oil Return

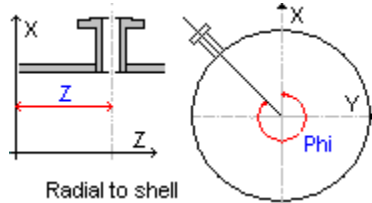
14 Jan. 2021 10:38 ConnID:S1.1

INPUT DATA

COMPONENT ATTACHMENT/LOCATION

Attachment: S1.1 Cylindrical Shell Shell

Connect this nozzle to the nozzle neck of another nozzle: NO



Orientation & Location of Nozzle: Radial to Shell

z-location of nozzle along axis of attachment.....:z 105.00 mm

Angle of Rotation of nozzle axis projected in the x-y plane:Phi 90.00 Degr.

GENERAL DESIGN DATA



Type of Opening: Nozzle Without Standard ASME or DIN/EN Flange Attachment
PRESSURE LOADING: Design Component for Internal Pressure Only

PROCESS CARD:

General Design Data : Temp= 100°C, P=13.0000 MPa, c=0.5 mm, Pext=0.0000 MPa

SPECIFIC DENSITY OF OPERATING LIQUID.....:SG 1.2000

LIQUID HEAD.....:LH 195.00 mm

Apply a different corrosion allowance to nozzle neck than the shell thickness.: NO

Include Nozzle Load Calculation: NO

SHELL DATA (S1.1)

Shell Type: Cylindrical Shell

OUTSIDE DIAMETER OF SHELL.....:De 168.30 mm

NOMINAL WALL THICKNESS (uncorroded).....:en 7.1000 mm

NEGATIVE TOLERANCE/THINNING ALLOWANCE.....:th 0.7100 mm

EN 10217-3:2002/A1:05, 1.0565 P355NH welded tube, HT:N THK<=20mm 100°C

Rm=490 Rp=355 Rpt=304 fs=202.67 f20=204.17 ftest=338.1 E=206067(N/mm²) ro=7.85

NOZZLE MATERIAL DATA



Delivery Form: Seamless Pipe

EN 10216-2:2013, 1.0425 P265GH seamless tube, HT:N THK<=16mm 100°C

Rm=410 Rp=265 Rpt=226 fb=150.67 f20=170.83 ftest=252.38 E=206067(N/mm²) ro=7.85

NOZZLE DIMENSIONAL DATA

Company Name -

Client :GÜVEN SOGUTMA

Vessel Tag No.:OS.C.130B.35.1

Visual Vessel Design by Hexagon PPM,Ver:20.0 Operator : Rev.:A

EN13445:2014 Issue 5:2018+A5 - 9.5 ISOLATED OPENINGS IN SHELLS

N.4 Oil Return

14 Jan. 2021 10:38 ConnID:S1.1



Attachment: Set In Flush Nozzle

Shape of Nozzle/Opening: Circular

Application:

9.4.6.3 NOT a critical fatigue area, and calc.temp.is outside creep range.

OUTSIDE NOZZLE DIAMETER.....:deb 20.00 mm

NOMINAL NOZZLE THICKNESS (uncorroded).....:enb 5.5000 mm

Size of Flange and Nozzle:

Comment (Optional):

NEGATIVE TOLERANCE/THINNING ALLOWANCE.....: 12.50 %

NOZZLE STANDOUT MEASURED FROM VESSEL OD.....:ho 23.00 mm

WELDING DATA

Nozzle/Pad to Shell Welding Area: User Specified Fillet Weld Throat Dimensions

OUTWARD NOZZLE WELD, THROAT DIMENSION.....:mo 3.5000 mm

Nozzle Weld Intersect: Nozzle Does NOT Intersect with a Welded Shell Seam

ANGLE PhiC(OBLIQUE IN TRANSVERSE.CROSS SECT.)Fig.9.5-2:PhiC 0.00 Degr.

ANGLE PhiL(OBLIQUE IN LONG.CROSS SECT.)Fig.9.5-1.....:PhiL 0.00 Degr.

DATA FOR REINFORCEMENT PAD



Type of Pad: No Pad

LIMITS OF REINFORCEMENT

Reduction of Limits of Reinforcement: No Reduction Required

WELDING REQUIREMENTS TO EN 1708-1:2010

Comment(Optional):

Type of welded connection: Not Applicable

CALCULATION DATA

PRELIMINARY CALCULATIONS

Shell Analysis Thickness eas
 $eas = en - c - th = 7.1 - 0.5 - 0.71 = 5.8900$ mm

Nozzle Analysis Thickness eab
 $eab = enb - cn - NegDev = 5.5 - 0.5 - 0.6875 = 4.3125$ mm

Inside Radius of Curvature
 $ris = De / 2 - eas (9.5-3) = 168.3 / 2 - 5.89 = 78.26$ mm

$dib = deb - 2 * eab = 20 - 2 * 4.3125 = 11.38$ mm

Min.Nozzle Thk.Based on Internal Pressure ebp

$ebp = P * deb / (2 * fb * z + P) = 13. * 20 / (2 * 150.67 * 1 + 13.) = 0.8300$ mm

Allowable Stresses

$fob = Min(fs, fb) (9.5-8) = Min(202.67, 150.67) = 150.67$ N/mm²

GEOMETRIC LIMITATIONS

»Check Max.Diameter of Nozzle $dib / (2 * ris) = 0.0727 <= 1.00 = 1[mm]$ «» OK«

Min.Nozzle Thk. $ebp = 0.83 <= eab = 4.3125[mm]$

19.2%

OK

9.5.2.4.4 Nozzles normal to the shell, with or without reinforcement pads.

Calculation of Stress Loaded Areas Effective as Reinforcement

Company Name -

Client :GÜVEN SOGUTMA

Vessel Tag No.:OS.C.130B.35.1

Visual Vessel Design by Hexagon PPM,Ver:20.0 Operator : Rev.:A

EN13445:2014 Issue 5:2018+A5 - 9.5 ISOLATED OPENINGS IN SHELLS

N.4 Oil Return

14 Jan. 2021 10:38 ConnID:S1.1

Area of Shell Afs

Limit of Reinforcement Along Shell

 $Iso = \text{Sqr}((2 * ris + eas) * eas)$ $= \text{Sqr}((2 * 78.26 + 5.89) * 5.89) =$

30.93 mm

Set In Nozzle

 $Afs = eas * Iso (9.5-79) = 5.89 * 30.93 =$ 182.17 mm²

Area of Nozzle Afb

Limit of Reinforcement Along Nozzle (outside shell)

 $Ibo = \text{MIN}(\text{Sqr}((deb - eb) * eb), ho)$

(9.5-76)

 $= \text{MIN}(\text{Sqr}((20 - 4.3125) * 4.3125), 23) =$

8.2251 mm

Set In Nozzle

 $Afb = eb * (Ibo + Ibi + eas) (9.5-78) = 4.3125 * (8.23 + 0 + 5.89) =$ 60.87 mm²

Area of Welds Afw

 $Afw = mo^2 = 3.5^2 =$ 12.25 mm²

Calculation of Pressure Loaded Areas

In the Nozzle Apb

 $Apb = 0.5 * dib * (Ibo + eas) (9.5-84) = 0.5 * 11.375 * (8.23 + 5.89) =$ 80.28 mm²

Cyl.Shell in the Longitudinal Section ApsL

 $ApsL = ris * (Is + a) (9.5-94) = 78.26 * (30.93 + 10) =$ 3203.09 mm²

Cyl.Shell in the Transverse Cross Section ApsT

 $ApsT = 0.5 * ris^2 * (Is + a) / (0.5 * eas + ris)$

(9.5-105)

 $= 0.5 * 78.26^2 * (30.93 + 10.03) / (0.5 * 5.89 + 78.26) =$ 1544.42 mm² $Aps = \text{MAX}(ApsL, ApsT) = \text{MAX}(3203.09, 1544.42) =$ 3203.09 mm²

9.5.2 Reinforcement Rules

Pressure Area Required pA(req.)

 $pAReqL = P * (ApsL + Apb) (9.5-7) = 13. * (3203.09 + 80.28) =$ 42.69 kN $pAReqT = P * (ApsT + Apb + 0.5 * Apphi)$

(9.5-7)

 $= 13. * (1544.42 + 80.28 + 0.5 * 0) =$ 21.12 kN $pAReq = \text{MAX}(pAReqL, pAReqT) = \text{MAX}(42691.4, 21124.89) =$ 42.69 kN

Pressure Area Available pA(aval.)

 $pAAval = (Afs + Afw) * (fs - 0.5 * P) + Afp * (fop - 0.5 * P) + Afb * (fob - 0.5 * P)$

(9.5-7)

 $= (182.17 + 12.25) * (202.67 - 0.5 * 13.) + 0 * (0 - 0.5 * 13.) + 60.87 * (150.67 - 0.5 * 13.)$ $= 46.92 \text{ kN}$ **Nozzle Reinforcement pAAval=46.92 >= pAReq=42.69[kN]****90.9%****OK**

Maximum Allowable Pressure Pmax

 $Pmax = (Afs + Afw) * fs + Afb * fob / ((ApsL + Apb) + 0.5 * (Afs + Afw + Afb + Afp))$

(9.5-10)

 $= (182.17 + 12.25) * 202.67 + 60.87 * 150.67 / ((3203.09 + 80.28) + 0.5 * (182.17 + 12.25 + 60.87 + 0)) =$ 14.24 MPa

Max.Allowable Test Pressure Pmax

 $Ptmax = ==$ 25.74 MPa

Weight of Nozzle: .0605kg

CALCULATION SUMMARY

Min.Nozzle Thk. ebp=0.83 <= eab=4.3125[mm]**19.2%****OK**

Company Name -

Client :GÜVEN SOGUTMA

Vessel Tag No.:OS.C.130B.35.1

Visual Vessel Design by Hexagon PPM,Ver:20.0 Operator : Rev.:A

EN13445:2014 Issue 5:2018+A5 - 9.5 ISOLATED OPENINGS IN SHELLS

N.4 Oil Return

14 Jan. 2021 10:38 ConnID:S1.1

9.5.2.4 Nozzles normal to the shell, with or without reinforcement pads.

Limit of Reinforcement Along Shell

$$Iso = \text{Sqr}((2 * ris + eas) * eas)$$

$$= \text{Sqr}((2 * 78.26 + 5.89) * 5.89) =$$

30.93 mm

Limit of Reinforcement Along Nozzle (outside shell)

$$Ibo = \text{MIN}(\text{Sqr}((deb - eb) * eb), ho)$$

(9.5-76)

$$= \text{MIN}(\text{Sqr}((20 - 4.3125) * 4.3125), 23) =$$

8.2251 mm

Pressure Area Required pA(req.)

$$pAreqL = P * (ApsL + Apb) \quad (9.5-7) = 13. * (3203.09 + 80.28) =$$

42.69 kN

$$pAreqT = P * (ApsT + Apb + 0.5 * Apphi)$$

(9.5-7)

$$= 13. * (1544.42 + 80.28 + 0.5 * 0) =$$

21.12 kN

$$pAreq = \text{MAX}(pAreqL, pAreqT) = \text{MAX}(42691.4, 21124.89) =$$

42.69 kN

Pressure Area Available pA(aval.)

$$pAAval = (Afs + Afw) * (fs - 0.5 * P) + Afp * (fop - 0.5 * P) + Afb * (fob - 0.5 * P) \quad (9.5-7)$$

$$= (182.17 + 12.25) * (202.67 - 0.5 * 13.) + 0 * (0 - 0.5 * 13.) + 60.87 * (150.67 - 0.5 * 13.)$$

$$= 46.92 \text{ kN}$$

Nozzle Reinforcement pAAval=46.92 >= pAReq=42.69[kN]**90.9%****OK****Maximum Allowable Pressure Pmax**

$$Pmax = (Afs + Afw) * fs + Afb * fob / ((ApsL + Apb) + 0.5 * (Afs + Afw + Afb + Afp)) \quad (9.5-10)$$

$$= (182.17 + 12.25) * 202.67 + 60.87 * 150.67 / ((3203.09 + 80.28) + 0.5 * (182.17 + 12.25 + 60.87 + 0)) =$$

14.24 MPa

Volume:0.00 m3 Weight:0.1 kg (SG= 7.85)

Company Name -

Client :GÜVEN SOGUTMA

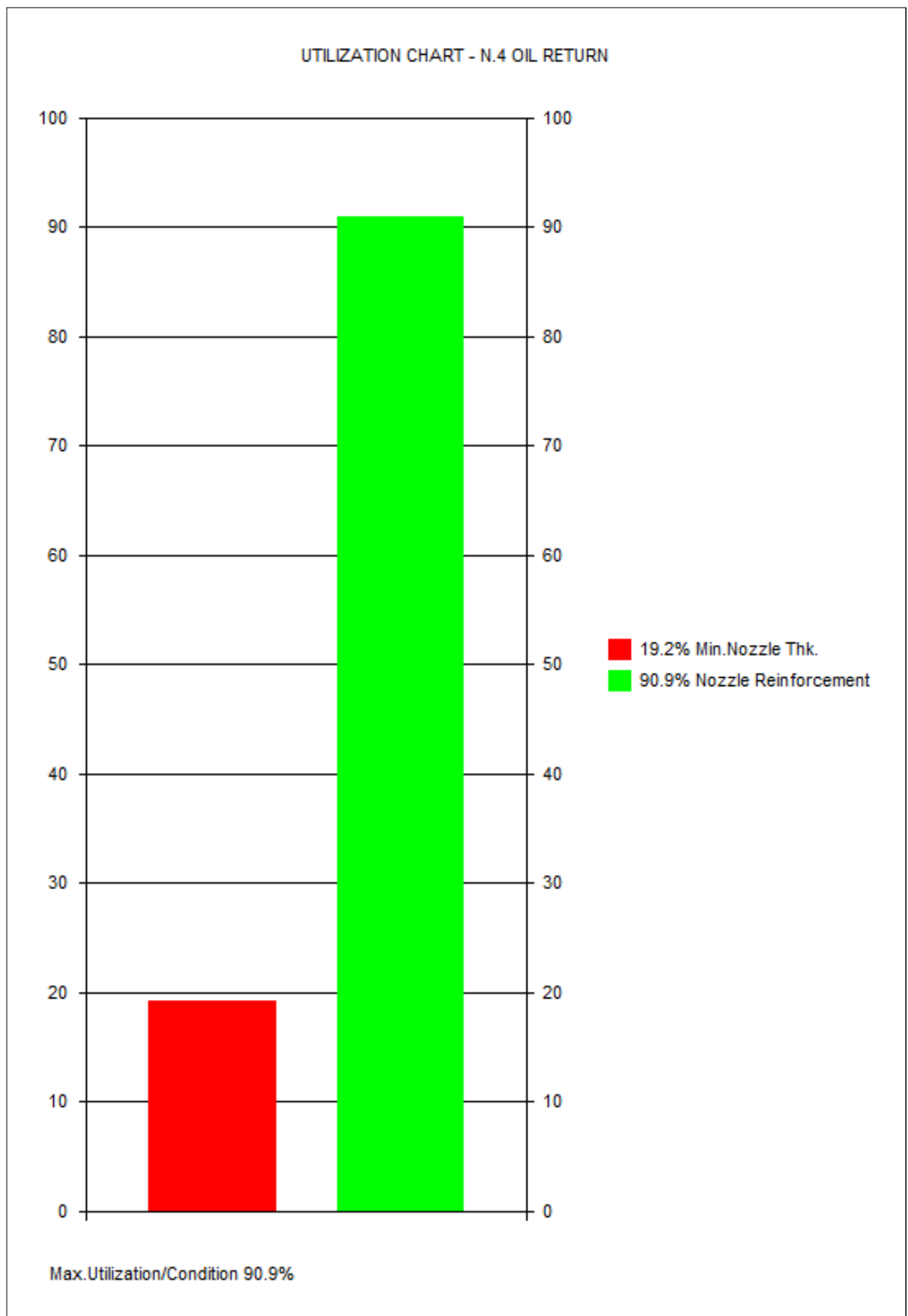
Vessel Tag No.:OS.C.130B.35.1

Visual Vessel Design by Hexagon PPM,Ver:20.0 Operator : Rev.:A

EN13445:2014 Issue 5:2018+A5 - 9.5 ISOLATED OPENINGS IN SHELLS

N.4 Oil Return

14 Jan. 2021 10:38 ConnID:S1.1



Company Name -

Client :GÜVEN SOGUTMA

Vessel Tag No.:OS.C.130B.35.1

Visual Vessel Design by Hexagon PPM,Ver:20.0 Operator : Rev.:A

EN13445:2014 Issue 5:2018+A5 - 11.10 FULL FACE FLANGES WITH METAL TO METAL C_
F.2 CT

INPUT DATA

COMPONENT ATTACHMENT/LOCATION

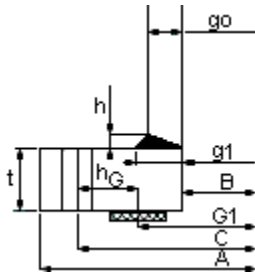
Attachment: S1.1 Cylindrical Shell Shell
Location: Along z-axis z1= 645
Flange Design Method: Section 11 - Taylor Forge

GENERAL DESIGN DATA

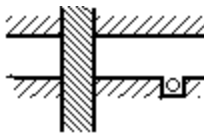
PROCESS CARD: General Design Data : Temp= 100°C, P=13.0000 MPa, c=0.5 mm
SPECIFIC DENSITY OF OPERATING LIQUID.....:SG 1.2000
LIQUID HEAD.....:LH 0.00 mm
B: Pressure loading: Flange under internal pressure
EXTERNAL LOADS ON FLANGE (PD5500 ENQ 5500/123): NO
SPECIFY BOLT LOADS FROM 2nd./MATING FLANGE: NO

TYPE OF FLANGE AND GASKET FACING

A: Flange Standard: User Specified Flanges



C: Flange Type: RT Ring Type(Smooth or Stepped bore)



D: Flange Facing (Sketch/Description):

7 Full faced flanges with metal to metal contact(O-rings)

SHELL/NOZZLE DATA

SHELL/NOZZLE SIZE & COMMENT: S1.1
EN 10217-3:2002/A1:05, 1.0565 P355NH welded tube, HT:N THK<=20mm 100'C
Rm=490 Rp=355 Rpt=304 fs=202.67 fs20=204.17 ftest=338.1 E=206067(N/mm²) ro=7.85
OUTSIDE DIAMETER OF SHELL/NOZZLE:Do 168.30 mm
WALL THICKNESS OF NOZZLE/SHELL(uncorroded).....:s1 7.1000 mm

FLANGE DATA

FLANGE HUB: Flange With Hub
DESIGN METHOD: A) INTEGRAL FLANGE METHOD
FLANGE BORE: Stepped
INSIDE DIAMETER OF FLANGE corroded.....:B 161.00 mm
OUTSIDE DIAMETER OF FLANGE.....:A 223.00 mm
THICKNESS OF FLANGE(uncorroded).....:e 32.00 mm
THICKNESS OF FLANGE AT REDUCED SECTION.....:er 27.00 mm
CORROSION ALLOWANCE FOR FLANGE FACE.....:cf 0.00 mm
EN 10028-2:2017, 1.0473 P355GH plate and strip, HT:N THK<=40mm 100'C
Rm=510 Rp=345 Rpt=314 SFO=209.33 SFA=212.5 ftest=328.57 E=206067(N/mm²) ro=7.85

Company Name -

Client :GÜVEN SOGUTMA

Vessel Tag No.:OS.C.130B.35.1

Visual Vessel Design by Hexagon PPM,Ver:20.0 Operator : Rev.:A

EN13445:2014 Issue 5:2018+A5 - 11.10 FULL FACE FLANGES WITH METAL TO METAL C_
F.2 CT

DATA FOR FLANGE HUB

LENGTH OF HUB.....:h 5.0000 mm
THICKNESS OF HUB AT BACK OF FLANGE corroded.....:g1 10.00 mm
THICKNESS OF HUB AT SMALL END corroded.....:go 5.0000 mm
EN 10028-2:2017, 1.0473 P355GH plate and strip, HT:N THK<=16mm 100'C
Rm=510 Rp=355 Rpt=323 SHO=212.5 SHA=212.5 ftest=338.1 E=206067(N/mm2) ro=7.85

BOLTING DATA

BOLTING TORQUE CALCULATION: NO
NOMINAL BOLTING SIZE & COMMENT: M14x2 ;
EFFECTIVE BOLT AREA per bolt.....:Ae 115.00 mm2
RECOMMENDED MINIMUM BOLT CENTER TO EDGE CLEARANCE...:Bce 17.50 mm
RECOMMENDED MINIMUM BOLT CENTER/RADIAL CLEARANCE....:Bcr 21.50 mm
DIAMETER OF BOLT HOLES IN FLANGE.....:d 16.00 mm
NUMBER OF BOLTS.....:n 16.00
BOLT-CIRCLE DIAMETER.....:C 195.00 mm
EN 10269:2013, 2.4668 NiCr19Fe19Nb5Mo3 bar, bolt, HT:P THK<=160mm 100'C
Rm=1230 Rp=1030 Rpt=1000 Sb=307.5 Sa=307.5 ftest=461.25 E=193180(N/mm2) ro=7.93

GASKET DATA

Table H-1 Gasket factors m & y Facing:
Rubber O-rings 75-85 BS and IRH m=0.25 Y=1.4 2 7
DIAMETER AT LOCATION OF GASKET LOAD REACTION.....:G 161.00 mm

WELDING REQUIREMENTS TO EN 1708-1:2010

Comment(Optional):
Type of welded connection: Not Applicable

CALCULATION DATA

FLANGE LOADS

HD = 0.785 * B ^ 2 * p =0.785*161^2*13= 264.52 kN
H = 0.785 * G ^ 2 * p =0.785*161^2*13= 264.52 kN
HT = H - HD =2.6452E05-2.6452E05= 0.00 kN

MOMENT ARMS

hD = (C - B - g1) / 2 =(195-161-10)/2= 12.00 mm
hT = (2 * C - G - B) / 4 =(2*195-161-161)/4= 17.00 mm
hR = (A - C) / 2 (11.10-1) =(223-195)/2= 14.00 mm

FLANGE MOMENTS

MR = HD * hD + HT * hT (11.10-2) =2.6452E05*12+0*17= 3174.29 Nm
HR = MR / hR (11.10-3) =3174.29/14= 226.73 kN

BOLT LOADS

Operating condition
Wop = H + HR (11.10-5) =2.6452E05+2.2673E05= 491.26 kN
Bolting up condition
Wamb = 0 (11.10-4) =0= 0.00 kN

BOLTING AREA

Am1 = Wop / Sb =4.9126E05/307.5= 1597.59 mm2
Am2 = Wamb / Sa =0/307.5= 0.00 mm2
Required Bolting Area Am
Am = MAX(Am1 , Am2) =MAX(1597.59,0)= 1597.59 mm2
Available Bolting Area Ab
Ab (num.bolts*root area) = n * Ae =16*115= 1840.00 mm2

Bolting Area Check Ab=1840 >= Am=1597.59[mm2]

86.8%

OK

Company Name -

Client :GÜVEN SOGUTMA

Vessel Tag No.:OS.C.130B.35.1

Visual Vessel Design by Hexagon PPM,Ver:20.0 Operator : Rev.:A

EN13445:2014 Issue 5:2018+A5 - 11.10 FULL FACE FLANGES WITH METAL TO METAL C_
F.2 CT**BOLT SPACING**

Actual Bolt Pitch

$$\text{Boltp} = 2 * (C / 2) * \text{Sqr}(1 - (\text{Cos}(PI / n))^2)$$

$$= 2 * (195 / 2) * \text{Sqr}(1 - (\text{Cos}(3.14 / 16))^2) =$$

38.04 mm

MINIMUM FLANGE THICKNESS

Minimum Flange Thickness Excl. Corr.

$$e_{min} = \text{Sqr}(6 * MR / (f * (PI * C - n * d)))$$

$$= \text{Sqr}(6 * 3174.29 / (209.33 * (3.14 * 195 - 16 * 16))) =$$

(11.10-6)
15.97 mm

Required Flange Thk. e=27 >= emin=15.97[mm]

59.1%

OK

EN13445-5;10.2.3.3 REQUIRED MIN.HYDROSTATIC TEST PRESSURE:Ptmin

NEW AT AMBIENT TEMP. FOR TEST GROUPS 1, 2 and 3

$$Ptmin = 1.25 * Pd * f_{20} / f = 1.25 * 13 * 212.5 / 209.33 =$$

16.50 MPa

$$Ptmin = 1.43 * Pd = 1.43 * 13 =$$

18.59 MPa

Test Pressure Ptmin=18.59 <= Pmax=22.46[MPa]

82.7%

OK

PRESSURE AND TORQUE SUMMARY

Table PRESSURE AND TORQUE SUMMARY FOR F.2 :

Description	Temp(C)	P(MPa)	Limited By	Min.Req.Total Bolt Force(kN)
Design Pressure(corroded)	100	13.00	Bolting Area Check	491.26
Max.Allow.Pressure(corroded)	100	15.08	Bolting Area Check	566.08
Max.Allow.Pressure(corroded)	Ambient	15.08	Bolting Area Check	566.08
Max.Allow.Test Pressure(corroded)	Ambient	22.46	Bolting Area Check	848.77
Required Test Pressure	Ambient	16.50	Bolting Area Check	580.03

The nominal Force and Torque values are based on the following bolting up method:

CALCULATION SUMMARY**BOLTING AREA**Bolting Area Check Ab=1840 >= Am=1597.59[mm²]

86.8%

OK

MINIMUM FLANGE THICKNESS

Minimum Flange Thickness Excl. Corr.

$$e_{min} = \text{Sqr}(6 * MR / (f * (PI * C - n * d)))$$

$$= \text{Sqr}(6 * 3174.29 / (209.33 * (3.14 * 195 - 16 * 16))) =$$

(11.10-6)
15.97 mm

Required Flange Thk. e=27 >= emin=15.97[mm]

59.1%

OK

EN13445-5;10.2.3.3 REQUIRED MIN.HYDROSTATIC TEST PRESSURE:Ptmin

NEW AT AMBIENT TEMP. FOR TEST GROUPS 1, 2 and 3

$$Ptmin = 1.25 * Pd * f_{20} / f = 1.25 * 13 * 212.5 / 209.33 =$$

16.50 MPa

$$Ptmin = 1.43 * Pd = 1.43 * 13 =$$

18.59 MPa

Test Pressure Ptmin=18.59 <= Pmax=22.46[MPa]

82.7%

OK

Volume:0.0006991 m³ Weight:4 kg (SG= 7.85)

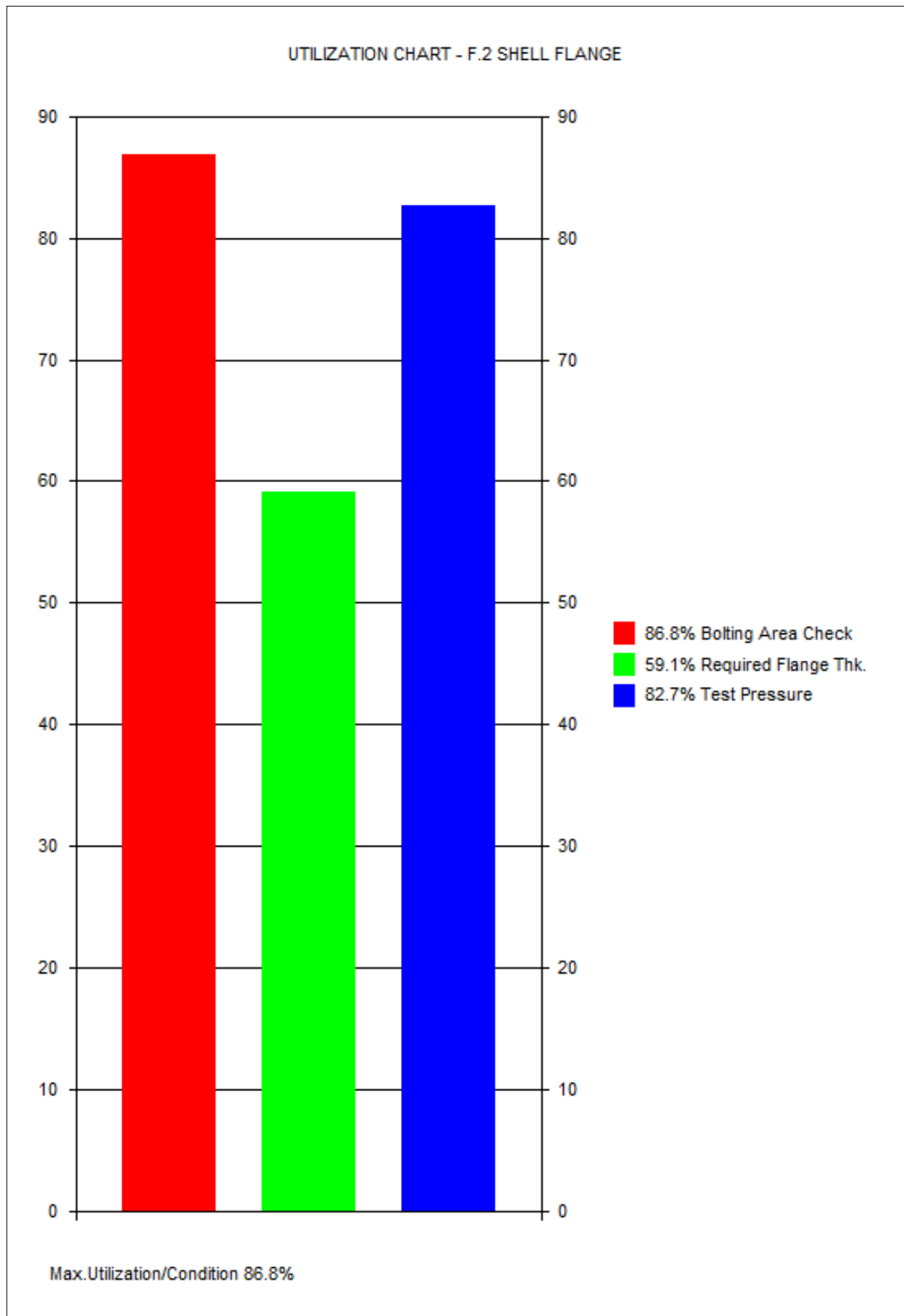
Company Name -

Client :GÜVEN SOGUTMA

Vessel Tag No.:OS.C.130B.35.1

Visual Vessel Design by Hexagon PPM,Ver:20.0 Operator : Rev.:A

EN13445:2014 Issue 5:2018+A5 - 11.10 FULL FACE FLANGES WITH METAL TO METAL C_
F.2 CT



Company Name -

Client :GÜVEN SOGUTMA

Vessel Tag No.:OS.C.130B.35.1

Visual Vessel Design by Hexagon PPM,Ver:20.0 Operator : Rev.:A

EN13445:2014 Issue 5:2018+A5 - EN13445; 16.12 - VERTICAL VESSELS WITH SKIRT

SK.1 Skirt 09 Feb. 2021 14:35 ConnID:S1.1

INPUT DATA

SKIRT DATA

Skirt is connected to component: S1.1
 SELECT TYPE OF SHAPE: Shape A - Skirt connection via support in cylinder area
 MEAN SKIRT DIAMETER AT TOP.....:Dz 114.00 mm
 MEAN SKIRT DIAMETER AT BOTTOM.....:Dzb 114.00 mm
 NOMINAL THICKNESS OF SKIRT.....:ez 3.0000 mm
 EN 10025-2:2005, 1.0038 S235JR Flat/Long Products THK<=16mm 50'C
 Rm=360 Rp=235 Rpt=235 fz=78.33 fz20=78.33 ftest=117.5 (N/mm2)
 NOTE: A PARTICULAR MATERIAL APPRAISAL(PMA) MAY BE REQUIRED FOR THIS MATERIAL.
 Select a different material for lower part of skirt: NO
 HEIGHT OF SUPPORT RING BETWN.SKIRT AND SHELL.....:h 0.00 mm
 MODULUS OF ELASTICITY at design temp.....:E 2,0607E05 N/mm2
 SAFETY FACTOR (1.0 carbon and 1.25 austenitic steels):s 1.0000

GENERAL LOAD DATA

Wind Load: YES
 Type of Wind Load: User Defined - Wind Velocity
 Wind Load Distribution: Evenly Wind Load Distribution
 MAXIMUM/PEAK WIND VELOCITY.....:Lw 10.00 m/s
 WIND FORCE/VESSEL SHAPE/DRAG COEFFICIENT.....:Cf 0.7000
 Check Wind Induced Vibration to EN13445 Section 22.10: YES
 Seismic Load: YES
 Type of Seismic Load: Uniform Building Code UBC 1997
 Seismic Zone Factor (Table 16-I): Zone 4, Z=0.4
 Site Coefficient for Soil Profile (Table 16-Q): SC Very Dense Soil and Soft Rock
 Nonbuilding Factor R (Table 16-P): Vertical Vessels on Legs, R=2.2
 OCCUPANCY IMPORTANCE COEFFICIENT (1.0 for vessels)..:I 1.0000
 VERTICAL SPECTRAL RESPONSE IN PERCENT OF HORIZONTAL.:vs 70.00 %
 Acceleration Loads: NO
 Blast Pressure Load: NO

EXTERNAL LOAD BEARING COMPONENTS

Table COMPONENTS:

Description	ID	Do1(mm)	Do2(mm)	L(mm)	Thk(mm)	z1(mm)	z2(mm)	A(m2)	Sp.Dens.
Shell	S1.1	168.3	168.3	645	7.1	0	645	0.11	7.85
Shell Flange	F.2	168.3	223	37	7.1	645	682	0.01	7.85
Cover Flange	E5.1	223	1	23	23	682	705	0	7.85

Table COMPONENTS Continued

Description	Weight(kg)	Vol(m3)	Material Name	fd	fa	fcd	fca	E-Module
Shell	18.2	0.012	EN 10217-3:2002/A1:05, 1.0565	202.7	204.2	191.1	221.9	206067.6
Shell Flange	4	0.001	EN 10028-2:2017, 1.0473 P355GH	209.3	212.5	194.8	213.1	206067.6
Cover Flange	7	0	EN 10028-2:2017, 1.0473 P355GH	209.3	212.5	202.4	221.9	206067.6

Table COMPONENTS Continued

Description	S	Thinning(mm)	E20-Module	Pemax
Shell	1	0.71	211770.7	0
Shell Flange	1	0	211770.7	0
Cover Flange	1	0	211770.7	0

DESIGN LOADS

Table DESIGN LOADS:

Load Description	ID	Fx-kN	Fy-kN	Fz-kN	x(mm)	y(mm)	z(mm)
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Company Name -

Client :GÜVEN SOGUTMA

Vessel Tag No.:OS.C.130B.35.1

Visual Vessel Design by Hexagon PPM,Ver:20.0 Operator : Rev.:A

EN13445:2014 Issue 5:2018+A5 - EN13445; 16.12 - VERTICAL VESSELS WITH SKIRT

SK.1 Skirt 09 Feb. 2021 14:35 ConnID:S1.1

LOAD CASES/COMBINATION

Table LOAD CASES:

Description	ID	LC9 Hydrotest	LC4 Shut Down	LC5 Installation	LC1&2&3 Oper.Wind
Wind Load	W	0.6	1.1	0.7	1.1
Seismic	S	0	0	0	0
Blast Load	B	0	0	0	0
Acceleration	A	0	0	0	0

Table LOAD CASES Continued

Description	LC6&7&8 Oper.Seismic
Wind Load	0
Seismic	1.0
Blast Load	0
Acceleration	0

LOAD CASE FACTORS

Table LOAD CASE FACTORS:

Description	ID	LC9 Hydrotest	LC4 Shut Down	LC5 Installation	LC1&2&3 Oper.Wind
Int.Pressure(MPa)	P	18.59	0	0	11.7
Ext.Pressure(MPa)	Pe	0	0	0	0
Temperature D/A	T	A	D	A	D
Corrosion (mm)	c	0	0.5	0.5	0.5
Stress M-Factor :	mf	1.425	1	1	1
Liquid Level (mm)	LL	FULL	EMPTY	EMPTY	300
Sp.Gravity (Liq.)	SG	1	0	0	1.2
Max.Deflection d/200	d	1	1	1	1

Table LOAD CASE FACTORS Continued

Description	LC6&7&8 Oper.Seismic
Int.Pressure(MPa)	11.7
Ext.Pressure(MPa)	0
Temperature D/A	D
Corrosion (mm)	0.5
Stress M-Factor :	1.425
Liquid Level (mm)	300
Sp.Gravity (Liq.)	1.2
Max.Deflection d/200	1

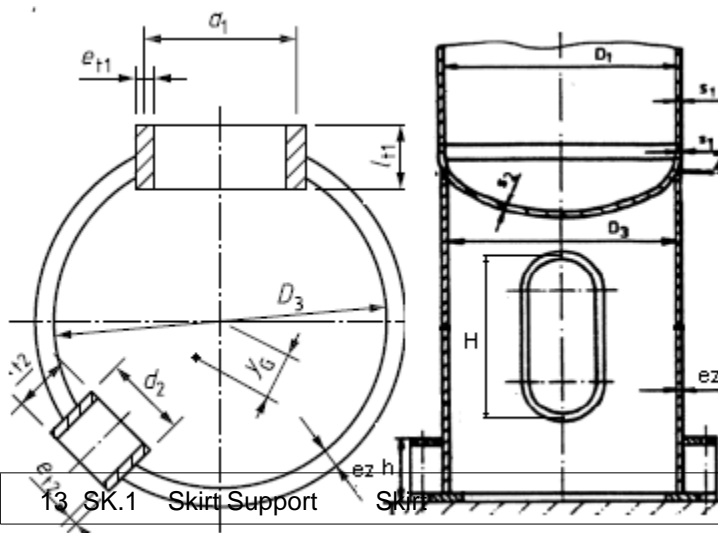
SKIRT OPENINGS

Table DATA FOR SKIRT OPENINGS:

Opening ID.	Dia./Width(mm)	Center z-value(mm)	Angle(degr.)	Height(H)(mm)
Inspection Opening	0	0	0	0

Table DATA FOR SKIRT OPENINGS Continued

Opening ID.	Reinf.thk.(eti)(mm)	Reinf.Length(lti)(mm)
Inspection Opening	0	



13 SK.1 Skirt Support

Umax= 75.1%

Page: 39

Company Name -

Client :GÜVEN SOGUTMA

Vessel Tag No.:OS.C.130B.35.1

Visual Vessel Design by Hexagon PPM,Ver:20.0 Operator : Rev.:A

EN13445:2014 Issue 5:2018+A5 - EN13445; 16.12 - VERTICAL VESSELS WITH SKIRT

SK.1 Skirt 09 Feb. 2021 14:35 ConnID:S1.1

VESSEL DATA

MEAN SHELL DIAMETER.....:DB 162.41 mm
THICKNESS OF VESSEL WALL (uncorroded).....:eB 5.8900 mm
EN 10217-3:2002/A1:05, 1.0565 P355NH welded tube, HT:N THK<=20mm 100'C
Rm=490 Rp=355 Rpt=304 fB=202.67 fB20=204.17 ftest=338.1 E=206067(N/mm2) ro=7.85

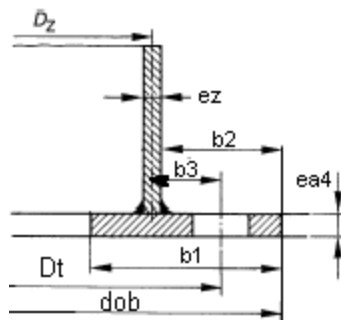
ANCHOR BOLT DATA

NUMBER OF BOLTS.....:n 4.0000
EFFECTIVE BOLT AREA per bolt.....:Ae 36.60 mm²
Bolting Size/Comment: M8x1.25 ;
5.6 -2 DIN 267 THK<=30mm 100'C
Rm=500 Rp=300 Rpt=250 Sa=83.33 Sb=100 ftest=150 (N/mm²)
NOTE: A PARTICULAR MATERIAL APPRAISAL(PMA) MAY BE REQUIRED FOR THIS MATERIAL.
Allowable Bolt Material Stress Based On:
Apply Safety Factors of 2.0625 on tensile- and 1.65 on yield-stress at room temp.

Allowable stresses for anchor bolts: Sa = 181.8, Sb = 181.8

ANCHOR BOLTS FRICTION COEFFICIENT: Normal/Average Conditions $\mu=0.20$
ANCHOR BOLTS ASSEMBLY FACTOR(RECOMMENDED VALUE 0.5):Phi 0.5000

DATA FOR SKIRT BASE



SELECT TYPE OF BASE: Skirt with base ring only
OUTSIDE DIAMETER OF BASE PLATE.....:dob 190.00 mm
NOMINAL THICKNESS OF BASE PLATE.....:ea4 5.0000 mm
TOTAL WIDTH OF BASE PLATE RING.....:b1 70.00 mm
DIAMETER OF BOLT HOLE IN BASE PLATE RING.....:dtb 8.5000 mm
Skirt base design temperature.....:Tb 100.00 'C
EN 10028-2:2017, 1.0345 P235GH plate and strip, HT:N THK<=16mm 100'C
Rm=360 Rp=235 Rpt=214 fc1=142.67 fc20=150 ftest=223.81 E=206067(N/mm2) ro=7.85
DIST.BETWEEN SKIRT OD AND CENTERLINE OF ANCHOR BOLTS:b3 23.00 mm
ALLOWABLE FOUNDATION BEARING PRESSURE.....:Fba 7.4000 N/mm²

WELDING REQUIREMENTS TO EN 1708-1:2010

Comment(Optional):
Type of welded connection: Not Applicable

CALCULATION DATA

Total Height of Unit
Height = ABS(zmax - zmin) =ABS(705--60)= 765.00 mm

Uniform Building Code 1997

Ca (from UBC Table 16Q) = == 0.4000
Cv (from UBC Table 16R) = == 0.5600
Fundamental period of vibration TRay calculated using the Rayleigh method:
TRay = $2 \cdot \pi \cdot \text{SQR}(\text{SUM}(W_i \cdot y_i^2) / (g \cdot \text{SUM}(W_i \cdot y_i)))$ where
Wi is the element weight, yi is the element deflection

Company Name -

Client :GÜVEN SOGUTMA

Vessel Tag No.:OS.C.130B.35.1

Visual Vessel Design by Hexagon PPM,Ver:20.0 Operator : Rev.:A

EN13445:2014 Issue 5:2018+A5 - EN13445; 16.12 - VERTICAL VESSELS WITH SKIRT

SK.1 Skirt 09 Feb. 2021 14:35 ConnID:S1.1

SEISMIC LOAD CASE NO: 5 - LC6&7&8 OPER.SEISMIC

$$T_{nat} = \text{MIN}(T_{Ray}, C_z * T_{natA})$$

$$= \text{MIN}(0.0078, 1.3 * 0.0399) =$$

0.0078 s

The total design base shear is given by the following formulas (rigid vessel $T < 0.06$):

$$V = 0.7 * C_a * I * W (34-1) = 0.7 * 0.4 * 1 * 50.32 =$$

14.09 kg

Shear force at bottom of vessel V

$$V = V * 9.81 / 1.4 = 14.09 * 9.81 / 1.4 =$$

0.0987 kN

$F_t = 0$ since $T < 0.7$ s

VESSEL DEFLECTION LOAD CASE NO: 1 - LC9 HYDROTEST

ID	z(mm)	l(m4)	E-Mod.(N/mm2)	F Shear(kN)	Moment(kNm)	Deflection(mm)
SK	-60	0.6366	211771	0.00	0.002	0.000
S1.1	0	0.1170	211771	0.00	0.000	0.000
S1.1	0	0.1170	211771	0.00	0.002	0.000
F.2	645	0.1170	211771	0.00	0.000	0.000
E5.1	682	1.2139	211771	0.00	0.000	0.000

»LC9 HYDROTEST, Max.deflection over 200mm length $d_{allow}=1$ \geq $d_{actual}=6.259E-05$ [mm] «»

OK«

Between $z_1=508$ and $z_2=705$ in component:E5.1

Deflection at top of vessel : 0.0 mm

VESSEL DEFLECTION LOAD CASE NO: 2 - LC4 SHUT DOWN

ID	z(mm)	l(m4)	E-Mod.(N/mm2)	F Shear(kN)	Moment(kNm)	Deflection(mm)
SK	-60	0.6366	206070	0.01	0.003	0.000
S1.1	0	0.1098	206068	0.00	0.000	0.000
S1.1	0	0.1098	206068	0.01	0.002	0.000
F.2	645	0.1098	206068	0.00	0.000	0.000
E5.1	682	1.2139	206068	0.00	0.000	0.000

»LC4 SHUT DOWN, Max.deflection over 200mm length $d_{allow}=1$ \geq $d_{actual}=1.0323E-04$ [mm] «»

OK«

Between $z_1=484$ and $z_2=682$ in component:F.2

Deflection at top of vessel : 0.0 mm

VESSEL DEFLECTION LOAD CASE NO: 3 - LC5 INSTALLATION

ID	z(mm)	l(m4)	E-Mod.(N/mm2)	F Shear(kN)	Moment(kNm)	Deflection(mm)
SK	-60	0.6366	211771	0.00	0.002	0.000
S1.1	0	0.1098	211771	0.00	0.000	0.000
S1.1	0	0.1098	211771	0.00	0.002	0.000
F.2	645	0.1098	211771	0.00	0.000	0.000
E5.1	682	1.2139	211771	0.00	0.000	0.000

»LC5 INSTALLATION, Max.deflection over 200mm length $d_{allow}=1$ \geq $d_{actual}=7.1617E-05$ [mm] «»

OK«

Between $z_1=508$ and $z_2=705$ in component:E5.1

Deflection at top of vessel : 0.0 mm

VESSEL DEFLECTION LOAD CASE NO: 4 - LC1&2&3 OPER.WIND

ID	z(mm)	l(m4)	E-Mod.(N/mm2)	F Shear(kN)	Moment(kNm)	Deflection(mm)
SK	-60	0.6366	206070	0.01	0.003	0.000
S1.1	0	0.1098	206068	0.00	0.000	0.000
S1.1	0	0.1098	206068	0.01	0.002	0.000
F.2	645	0.1098	206068	0.00	0.000	0.000
E5.1	682	1.2139	206068	0.00	0.000	0.000

»LC1&2&3 OPER.WIND, Max.deflection over 200mm length $d_{allow}=1$ \geq $d_{actual}=1.0323E-04$ [mm]

«» OK«

Between $z_1=484$ and $z_2=682$ in component:F.2

Deflection at top of vessel : 0.0 mm

Company Name -

Client :GÜVEN SOGUTMA

Vessel Tag No.:OS.C.130B.35.1

Visual Vessel Design by Hexagon PPM,Ver:20.0 Operator : Rev.:A

EN13445:2014 Issue 5:2018+A5 - EN13445; 16.12 - VERTICAL VESSELS WITH SKIRT

SK.1 Skirt 09 Feb. 2021 14:35 ConnID:S1.1

VESSEL DEFLECTION LOAD CASE NO: 5 - LC6&7&8 OPER.SEISMIC

ID	z(mm)	l(m4)	E-Mod.(N/mm2)	F Shear(kN)	Moment(kNm)	Deflection(mm)
SK	-60	0.6366	206070	0.10	0.054	0.000
S1.1	0	0.1098	206068	0.00	0.000	0.000
S1.1	0	0.1098	206068	0.10	0.050	0.000
F.2	645	0.1098	206068	0.04	0.002	0.006
E5.1	682	1.2139	206068	0.03	0.000	0.007

»LC6&7&8 OPER.SEISMIC, Max.deflection over 200mm length dallow=1 >= dactual=0.0023[mm] «»

OK«

Between z1= 484 and z2= 682 in component:F.2

Deflection at top of vessel : 0.0 mm

Natural Frequency of Vessel

The natural frequency of vibration is based on Rayleighs method of approximation:

$T = 2\pi \sqrt{\sum(W_i \cdot y_i^2) / (g \cdot \sum(W_i \cdot y_i))}$; where

W_i is the weight of the i th. element and y_i is the deflection of this element.

LOAD CASE	Fundamental Period(s)	Natural Frequency(Hz)
LOAD CASE NO: 1 - LC9 HYDROTEST	0.0086 s	116.63 Hz
LOAD CASE NO: 2 - LC4 SHUT DOWN	0.0078 s	128.88 Hz
LOAD CASE NO: 3 - LC5 INSTALLATION	0.0077 s	130.65 Hz
LOAD CASE NO: 4 - LC1&2&3 OPER.WIND	0.0078 s	127.98 Hz
LOAD CASE NO: 5 - LC6&7&8 OPER.SEISMIC	0.0078 s	127.98 Hz

EN13445 Section 22.10.2 - Criteria for Vortex Shedding

The effect of vortex shedding need not be investigated when at least one of the following conditions is met:

$h/Dc13 < 15$, $V_{cr} > 1.25 \cdot v_m$, $0.004 \cdot m_e / (\rho \cdot Dc13^2) > 0.8$ for oper.cond and 1.1 for erection cond.

LOAD CASE	Dc13(mm)	me(kg/m)	Vm(m/s)	Vcr(m/s)	h/Dc13	$0.004 \cdot m_e / (\rho \cdot Dc13^2)$	Status
NO: 1 - LC9 HYDROTEST	167.19	90.72	10.00	108.33	4.58	10.18	OK
NO: 2 - LC4 SHUT DOWN	167.19	69.34	10.00	119.71	4.58	7.78	OK
NO: 3 - LC5 INSTALLATION	167.19	69.34	10.00	121.36	4.58	7.78	OK
NO: 4 - LC1&2&3 OPER.WIND	167.19	69.34	10.00	118.87	4.58	7.78	OK

Nomenclature:

h = Total height above ground level =0.7650 m

$Dc13$ = Average diameter(including insulation) of upper third of height of column.

m_e = Equivalent mass per unit length of the upper third of height of column.

v_{crit} = Critical wind velocity for mode 1.

v_m = 10 minute mean wind velocity with a 0.02 annual risk of being exceeded at elevation 0.6375 m above ground level.

22.10 Vortex Shedding - Forces and Moments

Load Case	Sc	K	Kw	clat	yF,max(mm)	Force(kN)	Moment(kNm)
LC9 Hydrotest	203.58	0.1300	0.6000	0.00	0.00	0.00	0.00
LC4 Shut Down	46.68	0.1300	0.6000	0.00	0.00	0.00	0.00
LC5 Installation	46.68	0.1300	0.6000	0.00	0.00	0.00	0.00
LC1&2&3 Oper.Wind	155.61	0.1300	0.6000	0.00	0.00	0.00	0.00

16.12.4.4 CHECK OF LARGEST OPENING IN SKIRT(at z= 0)

Half angle of the opening

$\Delta = \text{ArcSin}(d / D3) (16.12-72) = \text{ArcSin}(0/111) = 0.00 \text{ rad}$

Area of Reinforcement of opening

$A_t = 2 \cdot e_{ti} \cdot l_{ti} = 2 \cdot 0 \cdot 0 = 0.00 \text{ mm}^2$

Company Name -

Client :GÜVEN SOGUTMA

Vessel Tag No.:OS.C.130B.35.1

Visual Vessel Design by Hexagon PPM,Ver:20.0 Operator : Rev.:A

EN13445:2014 Issue 5:2018+A5 - EN13445; 16.12 - VERTICAL VESSELS WITH SKIRT

SK.1 Skirt 09 Feb. 2021 14:35 ConnID:S1.1

Area of Skirt

$As = (PI - \Delta) * D3 * ea3 = (3.14-0)*111*3=$ 1046.15 mm²
 Total Area of Available Skirt
 $A4 = As + At (16.12-73) = 1046.15+0=$ 1046.15 mm²
 $yt = 0.5 * D3 * \cos(\Delta) + ht - 0.5 * lti$
 $= 0.5*111*\cos(0)+0-0.5*0=$ 55.50 mm
 Distance between neutral axis and centre of gravity
 $yG = (0.5 * D3 * ea3 * d - 2 * lti * eti * yt) / A4$ (16.12-74)
 $= (0.5*111*3*0-2*0*0*55.5)/1046.15=$ 0.00 mm
 Maximum distance between centre of gravity and outer edge of section 4-4
 $y_{max} = \max(0.5 * D3 * \cos(\Delta) + ht + yG, 0.5 * D3 - yG)$
 $= \max(0.5*111*\cos(0)+0+0, 0.5*111-0)=$ 55.50 mm
 $Is = (PI-\Delta-\sin(\Delta))*\cos(\Delta)*ea3*(0.5*D3)^3$
 $= (3.14-0-\sin(0))*\cos(0)*3*(0.5*111)^3=$ 1,6112E06 mm⁴
 Section modulus of the cross section at section 4-4
 $W4 = (Is+At*(yt^2+lti^2/12)-A4*yG^2)/y_{max}$ (16.12-75)
 $= (1.6112E06+0*(55.5^2+0^2/12)-1046.15*0^2)/55.5=$ 29030.67 mm³
 Weakening Factor for Area
 $\psi_{s1} = \min(1, A4 / A_{unperf}) = \min(1, 1046.15/1074.42)=$ 0.9737
 Weakening Factor for Elastic Section Modulus
 $\psi_{s2} = \min(1, W4 / W_{unperf}) = \min(1, 29030.67/29856.61)=$ 0.9723

16.12.4 WEAK AREAS IN SKIRT DUE TO OPENING/MULTIPLE OPENINGS

Z-Loc.(mm)	Skirt Mean D(mm)	Area A4(mm ²)	Sect.Mod.W4(mm ³)	eps(mm)	No.Open.	psi1	psi2
-60	114	1074	29857	0.000	0	1.000	1.000
-20	114	1074	29857	0.000	0	1.000	1.000

eps = displacement of neutral axis at elevation z.

No.Open. = number of openings considered at elevation z.

psi1 = area reduction of skirt = actual area / area of unpierced skirt.

psi2 = section modulus reduction of skirt = actual value/value of unpierced skirt.

NOTE: Top and bottom of skirt is always checked as skirt diameter, skirt material, calculation temperature, allowable stress and moments may vary from top to bottom of skirt.

Note: Skirt is pierced and has a minimum section modulus at z= 0mm

REACTION FORCES & MOMENTS AT SKIRT BASE

LOAD CASE	Fx(kN)	Fy(kN)	Fz(kN)	Mx(kNm)	My(kNm)	Mz(kNm)
LC9 HYDROTEST	0.00	0.00	-0.51	0.00	0.00	0.00
LC4 SHUT DOWN	0.01	0.00	-0.37	0.00	0.00	0.00
LC5 INSTALLATION	0.00	0.00	-0.37	0.00	0.00	0.00
LC1&2&3 OPER.WIND	0.01	0.00	-0.43	0.00	0.00	0.00
LC6&7&8 OPER.SEISMIC	0.10	0.00	-0.50	0.00	0.05	0.00

LOAD CASE NO: 1 - LC9 HYDROTEST

Summation of Total Loads for Load Case : LC9 HYDROTEST

Fz (Force in Vertical Direction)= == -0.5128 kN
 Fx (Force in X-Direction)= == 0.0034 kN
 Fy (Force in Y-Direction)= == 0.00 kN
 $Fs = \sqrt{Fx^2 + Fy^2}$ (Tot.Force in Hor.Plane)= == 0.0034 kN
 Moments at top of skirt at elevation of shell/skirt connection.
 Mx (Moment around the X-Axis)= == -0.8814 kNm
 My (Moment around the Y-Axis)= == 1.3129 kNm
 $M1 = \sqrt{Mx^2 + My^2}$ (Moment in XY Plane)= == 1.5813 kNm
 Mt (Moment around the z-axis)= == 0.0128 kNm
 Moments at elevation at bottom of skirt.
 $Mx4$ (Moment around the X-Axis)= == -0.8814 kNm
 $My4$ (Moment around the Y-Axis)= == 1.4509 kNm
 $M4 = \sqrt{Mx4^2 + My4^2}$ (Moment in XY Plane)= == 1.6976 kNm
 FF (Weight of Content of Vessel)= == 0.1282 kN
 dFg (Weight of Vessel Below Section 2-2)= (16.12-3) == 2,7655E-04 kN

Company Name -

Client :GÜVEN SOGUTMA

Vessel Tag No.:OS.C.130B.35.1

Visual Vessel Design by Hexagon PPM,Ver:20.0 Operator : Rev.:A

EN13445:2014 Issue 5:2018+A5 - EN13445; 16.12 - VERTICAL VESSELS WITH SKIRT

SK.1 Skirt 09 Feb. 2021 14:35 ConnID:S1.1

16.12.6.1 Verification of Membrane Stresses

$$Fz_p = Fz + 4 * M1 / Dz \quad (16.12-1) = -512.8 + 4 * 1.58 / 114 = -457.29 \text{ N}$$

$$Fz_q = Fz - 4 * M1 / Dz \quad (16.12-2) = -512.8 - 4 * 1.58 / 114 = -568.26 \text{ N}$$

16.14.8 COMPRESSIVE STRESS LIMITS

$$K = 1.21 * E * ea / (Sige * D) \quad (16.14-15) = 1.21 * 2.1177E05 * 5.89 / (355 * 162.41) = 26.18$$

$$\alpha = 0.83 / \text{Sqr}(1 + 0.005 * D / ea) \quad (16.14-16) = 0.83 / \text{Sqr}(1 + 0.005 * 162.41 / 5.89) = 0.7781$$

$$\delta = (1 - 0.4123 / (\alpha * K) ^ 0.6) / S \quad (16.14-19) = (1 - 0.4123 / (0.7781 * 26.18) ^ 0.6) / 1.05 = 0.8880$$

Maximum Allowable Compressive Stress

$$\text{Sigcall} = Sige * \delta \quad (16.14-20) = 355 * 0.888 = 315.25 \text{ N/mm}^2$$

Stresses in section 1-1 (internal pressure)

Membrane Stress(Sect.1-1)

$$\text{Sig1pm} = (Fz_p + dFg + FF) / (\text{PI} * \text{DB} * eB) + p * \text{DB} / (4 * eB)$$

$$= (-457.29 + 0.2766 + 128.19) / (3.14 * 162.41 * 5.89) + 18.59 * 162.41 / (4 * 5.89)$$

$$= 128.04 \text{ N/mm}^2$$

$$\text{Sig1qm} = (Fz_q + dFg + FF) / (\text{PI} * \text{DB} * eB) + p * \text{DB} / (4 * eB)$$

$$= (-568.26 + 0.2766 + 128.19) / (3.14 * 162.41 * 5.89) + 18.59 * 162.41 / (4 * 5.89)$$

$$= 128.00 \text{ N/mm}^2$$

Membrane Stress(Pint)(Sect.1-1) Sig1pm=128.04 <= fB=338.1[N/mm2]	37.8%	OK
Membrane Stress(Pint)(Sect.1-1) ABS(Sig1qm)=128. <= fB=338.1[N/mm2]	37.8%	OK

Stresses in section 1-1 (external pressure)

Membrane Stress(Sect.1-1)

$$\text{Sig1pmext} = (Fz_p + dFg + FF) / (\text{PI} * \text{DB} * eB) - \text{Pext} * \text{DB} / (4 * eB)$$

$$= (-457.29 + 0.2766 + 128.19) / (3.14 * 162.41 * 5.89) - 0 * 162.41 / (4 * 5.89) = -0.1094 \text{ N/mm}^2$$

$$\text{Sig1qmext} = (Fz_q + dFg + FF) / (\text{PI} * \text{DB} * eB) - \text{Pext} * \text{DB} / (4 * eB)$$

$$= (-568.26 + 0.2766 + 128.19) / (3.14 * 162.41 * 5.89) - 0 * 162.41 / (4 * 5.89) = -0.1463 \text{ N/mm}^2$$

Compr.Stress(Pext)(Sect.1-1) Sig1pmext=0.1094 <= Sigcall=315.25[N/mm2]	0.0%	OK
Compr.Stress(Pext)(Sect.1-1) Sig1qmext=0.1463 <= Sigcall=315.25[N/mm2]	0.0%	OK

Membrane Stress(Sect.2-2)

$$\text{Sig2pm} = \text{Sig2qm} = (FF + dFg) / (\text{PI} * \text{DB} * eB) + p * \text{DB} / (4 * eB)$$

$$= (128.19 + 0.2766) / (3.14 * 162.41 * 5.89) + 18.59 * 162.41 / (4 * 5.89) = 128.19 \text{ N/mm}^2$$

Membrane Stress(Sect.2-2) ABS(Sig2pm)=128.19 <= fB=338.1[N/mm2]	37.9%	OK
---	-------	----

Membrane Stress(Sect.3-3)

$$\text{Sig3pm} = Fz_p / (\text{PI} * Dz * ez) = -457.29 / (3.14 * 114 * 3) = -0.4256 \text{ N/mm}^2$$

$$\text{Sig3qm} = Fz_q / (\text{PI} * Dz * ez) = -568.26 / (3.14 * 114 * 3) = -0.5289 \text{ N/mm}^2$$

Membrane Stress(Sect.3-3) ABS(Sig3pm)=0.4256 <= fz=117.5[N/mm2]	0.3%	OK
Membrane Stress(Sect.3-3) ABS(Sig3qm)=0.5289 <= fz=117.5[N/mm2]	0.4%	OK

Company Name -

Client :GÜVEN SOGUTMA

Vessel Tag No.:OS.C.130B.35.1

Visual Vessel Design by Hexagon PPM,Ver:20.0 Operator : Rev.:A

EN13445:2014 Issue 5:2018+A5 - EN13445; 16.12 - VERTICAL VESSELS WITH SKIRT

SK.1 Skirt 09 Feb. 2021 14:35 ConnID:S1.1

16.12.6.2 Bending Stresses

a) Structural Shape A - figure 16.12-1

Total Bending Moments at Verification Points p and q:

$$M_p = 0.5 * (D_z - DB) * F_{zp} \text{ (16.12-18)} = 0.5 * (114 - 162.41) * -457.29 = 11068.67 \text{ Nmm}$$

$$M_q = 0.5 * (D_z - DB) * F_{zq} \text{ (16.12-19)} = 0.5 * (114 - 162.41) * -568.26 = 13754.70 \text{ Nmm}$$

$$e_{1pm} = ((F_{zp} + dF_g + FF) / (PI * DB) + p * DB / 4) / f_B \text{ (16.12-7)}$$

$$= ((-457.29 + 0.2766 + 128.19) / (3.14 * 162.41) + 18.59 * 162.41 / 4) / 338.1 = 2.2306 \text{ mm}$$

$$e_{1qm} = ((F_{zq} + dF_g + FF) / (PI * DB) + p * DB / 4) / f_B \text{ (16.12-8)}$$

$$= ((-568.26 + 0.2766 + 128.19) / (3.14 * 162.41) + 18.59 * 162.41 / 4) / 338.1 = 2.2299 \text{ mm}$$

$$e_{2m} = ((dF_g + FF) / (PI * DB) + p * DB / 4) / f_B \text{ (16.12-11)}$$

$$= ((0.2766 + 128.19) / (3.14 * 162.41) + 18.59 * 162.41 / 4) / 338.1 = 2.2332 \text{ mm}$$

$$e_{3pm} = (F_{zp} / (PI * D_z)) / f_B \text{ (16.12-11)}$$

$$= (-457.29 / (3.14 * 114)) / 338.1 = -0.0038 \text{ mm}$$

$$e_{3qm} = (F_{zq} / (PI * D_z)) / f_B \text{ (16.12-11)}$$

$$= (-568.26 / (3.14 * 114)) / 338.1 = -0.0047 \text{ mm}$$

Total Section Modulus of the Support Ring at Verification Point

$$W_p = PI/4 * ((D_z + e_z - DB - e_B) * h^2 + (2 * e_B^2 - e_{1pm}^2 - e_{2m}^2) * DB + 0.5 * (e_{2m}^2 - e_{3pm}^2) * D_z) \text{ (16.12-20)}$$

$$= 3.14/4 * ((114 + 3 - 162.41 - 5.89) * 0^2 + (2 * 5.89^2 - 2.23^2 - 2.23^2) * 162.41 + 0.5 * (2.23^2 - (-0.0038)^2) * 114) = 7802.86 \text{ mm}^3$$

$$W_q = PI/4 * ((D_z + e_z - DB - e_B) * h^2 + (2 * e_B^2 - e_{1qm}^2 - e_{2m}^2) * DB + 0.5 * (e_{2m}^2 - e_{3qm}^2) * D_z) \text{ (16.12-21)}$$

$$= 3.14/4 * ((114 + 3 - 162.41 - 5.89) * 0^2 + (2 * 5.89^2 - 2.23^2 - 2.23^2) * 162.41 + 0.5 * (2.23^2 - (-0.0047)^2) * 114) = 7803.23 \text{ mm}^3$$

16.12.6.3 Total Stresses and Strength Conditions

a) Structural Shape A

1) Location p ABS(M_p)/ $W_p=1.42 \leq f_T=117.5$ [N/mm ²] (16.12-44)	1.2%	OK
2) Location q ABS(M_q)/ $W_q=1.76 \leq f_T=117.5$ [N/mm ²] (16.12-45)	1.5%	OK

16.12.7 Verification of Skirt (Section 4-4)

Note: Skirt is pierced and has a minimum section modulus at $z = 0$ mm

Verification of Skirt at Bottom(no openings) at Elevation $z = -60$ mm

Additional Moment due to Displacement of Neutral Axis

$$dM_4 = \epsilon_s * F_4 = 0 * 512.77 = 0.00 \text{ kNmm}$$

Membrane Stress(Sect.4-4/ $z=-60$)

$$\text{Sig}_{4pm} = (M_{44} + dM_4) / W_4 - F_4 / A_4 = (1697.62 + 0) / 29856.61 - 512.77 / 1074.43 = -0.4204 \text{ N/mm}^2$$

$$\text{Sig}_{4qm} = -1 * (M_{44} + dM_4) / W_4 - F_4 / A_4 = -1 * (1697.62 + 0) / 29856.61 - 512.77 / 1074.43 = -0.5341 \text{ N/mm}^2$$

Membrane Stress(Sect.4-4/ $z=-60$) $\text{Sig}_{4pm}=0.4204 \leq f_z=117.5$ [N/mm ²]	0.3%	OK
Membrane Stress(Sect.4-4/ $z=-60$) $\text{Sig}_{4qm}=0.5341 \leq f_z=117.5$ [N/mm ²]	0.4%	OK

Compressive Stresses in Skirt(Sect.4-4/ $z=-60$)

$$\text{Sig}_{4c} = -F_4 / A_4 = -512.77 / 1074.43 = -0.4773 \text{ N/mm}^2$$

16.14.8 COMPRESSIVE STRESS LIMITS

$$K = 1.21 * E * e_a / (\text{Sige} * D) \text{ (16.14-15)}$$

$$= 1.21 * 206070 * 3 / (235 * 114) = 27.92$$

$$\alpha = 0.83 / \text{Sqr}(1 + 0.005 * D / e_a) \text{ (16.14-16)}$$

$$= 0.83 / \text{Sqr}(1 + 0.005 * 114 / 3) = 0.7609$$

$$\Delta = (1 - 0.4123 / (\alpha * K))^{0.6} / S \text{ (16.14-19)}$$

$$= (1 - 0.4123 / (0.7609 * 27.92))^{0.6} / 1.05 = 0.8896$$

Maximum Allowable Compressive Stress

$$\text{Sig}_{call} = \text{Sige} * \Delta \text{ (16.14-20)} = 235 * 0.8896 = 209.06 \text{ N/mm}^2$$

Company Name -

Client :GÜVEN SOGUTMA

Vessel Tag No.:OS.C.130B.35.1

Visual Vessel Design by Hexagon PPM,Ver:20.0 Operator : Rev.:A

EN13445:2014 Issue 5:2018+A5 - EN13445; 16.12 - VERTICAL VESSELS WITH SKIRT

SK.1 Skirt 09 Feb. 2021 14:35 ConnID:S1.1

Compr.Stress Limits (4-4/z=-60) Sig4c=0.4773 <=
Sigcall=209.06[N/mm2]

0.2%

OK

Check of Skirt Stability/Weakening due to Openings in Skirt

Maximum Compressive Force at Section 4-4

$$F_{c4max} = \pi * D_4 * e_z * \text{Sigcall} \quad (16.14-2) = 3.14 * 114 * 3 * 209.06 = 224.62 \text{ kN}$$

Maximum Bending Moment at Section 4-4

$$M_{4max} = \pi / 4 * D_4^2 * e_z * \text{Sigcall} \quad (16.14-3) \\ = 3.14 / 4 * 114^2 * 3 * 209.06 = 6.4017 \text{ kNm}$$

Stability of skirt(Sect.4-4/z=-60)

$$\text{StabilityFactor} = F_4 / (\text{Psi1} * F_{c4max}) + (\text{ABS}(M_{44}) + F_4 * \text{eps}) / (\text{Psi2} * M_{4max}) \quad (16.12-70) \\ = 512.77 / (1 * 2.2462E05) + (\text{ABS}(1697.62) + 512.77 * 0) / (1 * 6.4017E06) = 0.0025$$

Stability/Openings in skirt(Sect.4-4/z=-60)
StabilityFactor=0.0025 <= 1=1

0.2%

OK

Verification of Skirt at Top(no openings) at Elevation z = -20 mm

Additional Moment due to Displacement of Neutral Axis

$$dM_4 = \text{eps} * F_4 = 0 * 512.77 = 0.00 \text{ kNm}$$

Membrane Stress(Sect.4-4/z=-20)

$$\text{Sig4pm} = (M_{44} + dM_4) / W_4 - F_4 / A_4 \\ = (1581.32 + 0) / 29856.61 - 512.77 / 1074.43 = -0.4243 \text{ N/mm}^2$$

Sig4qm = - 1 * (M44 + dM4) / W4 - F4 / A4

$$= -1 * (1581.32 + 0) / 29856.61 - 512.77 / 1074.43 = -0.5302 \text{ N/mm}^2$$

Membrane Stress(Sect.4-4/z=-20) Sig4pm=0.4243 <=
fz=117.5[N/mm2]

0.3%

OK

Membrane Stress(Sect.4-4/z=-20) Sig4qm=0.5302 <=
fz=117.5[N/mm2]

0.4%

OK

Compressive Stresses in Skirt(Sect.4-4/z=-20)

$$\text{Sig4c} = - F_4 / A_4 = -512.77 / 1074.43 = -0.4773 \text{ N/mm}^2$$

16.14.8 COMPRESSIVE STRESS LIMITS

$$K = 1.21 * E * e_a / (\text{Sige} * D) \quad (16.14-15) \\ = 1.21 * 206070 * 3 / (235 * 114) = 27.92$$

$$\text{alfa} = 0.83 / \text{Sqr}(1 + 0.005 * D / e_a) \quad (16.14-16) \\ = 0.83 / \text{Sqr}(1 + 0.005 * 114 / 3) = 0.7609$$

$$\text{delta} = (1 - 0.4123 / (\text{alfa} * K)^{0.6}) / S \quad (16.14-19) \\ = (1 - 0.4123 / (0.7609 * 27.92)^{0.6}) / 1.05 = 0.8896$$

$$\text{Maximum Allowable Compressive Stress} \\ \text{Sigcall} = \text{Sige} * \text{delta} \quad (16.14-20) = 235 * 0.8896 = 209.06 \text{ N/mm}^2$$

Compr.Stress Limits (4-4/z=-20) Sig4c=0.4773 <=
Sigcall=209.06[N/mm2]

0.2%

OK

Check of Skirt Stability/Weakening due to Openings in Skirt

Maximum Compressive Force at Section 4-4

$$F_{c4max} = \pi * D_4 * e_z * \text{Sigcall} \quad (16.14-2) = 3.14 * 114 * 3 * 209.06 = 224.62 \text{ kN}$$

Maximum Bending Moment at Section 4-4

$$M_{4max} = \pi / 4 * D_4^2 * e_z * \text{Sigcall} \quad (16.14-3) \\ = 3.14 / 4 * 114^2 * 3 * 209.06 = 6.4017 \text{ kNm}$$

Stability of skirt(Sect.4-4/z=-20)

$$\text{StabilityFactor} = F_4 / (\text{Psi1} * F_{c4max}) + (\text{ABS}(M_{44}) + F_4 * \text{eps}) / (\text{Psi2} * M_{4max}) \quad (16.12-70) \\ = 512.77 / (1 * 2.2462E05) + (\text{ABS}(1581.32) + 512.77 * 0) / (1 * 6.4017E06) = 0.0025$$

Stability/Openings in skirt(Sect.4-4/z=-20)
StabilityFactor=0.0025 <= 1=1

0.2%

OK

Company Name -

Client :GÜVEN SOGUTMA

Vessel Tag No.:OS.C.130B.35.1

Visual Vessel Design by Hexagon PPM,Ver:20.0 Operator : Rev.:A

EN13445:2014 Issue 5:2018+A5 - EN13445; 16.12 - VERTICAL VESSELS WITH SKIRT

SK.1 Skirt 09 Feb. 2021 14:35 ConnID:S1.1

SKIRT BASE DESIGN

Wsk = $F4 / (PI * Dz_b) = 512.77 / (3.14 * 114) = 1.4318$ N/mm
Msk = $4 * M4 / (PI * Dz_b^2) = 4 * 1.7 / (3.14 * 114^2) = 0.1663$ N/mm
Outer radial width of bearing plate/base plate
 $b2 = (dob - (Dz_b + ez)) / 2 = (190 - (114 + 3)) / 2 = 36.50$ mm

16.12.5 Anchor Bolt Design

Bolt Circle Diameter
 $d = Dz_b + ea3 + 2 * b3 = 114 + 3 + 2 * 23 = 163.00$ mm
Mean Diameter of Bearing Ring/Base Plate
DCR = $dob - b1 = 190 - 70 = 120.00$ mm
Inside Diameter of Bearing Ring/Base Plate
D3 = $dob - 2 * b1 = 190 - 2 * 70 = 50.00$ mm
Pre-tensioning of bolts FA
FA = $Phi * Ae * fdAnchor (16.12.80) = 0.5 * 36.6 * 181.82 = 3327.27$ N
Maximum anchor bolt force FB
FB = $(4 * M5 / d - F5) / n (16.12.77) = (4 * 1697.62 / 163 - 512.77) / 4 = -117.78$ N
Required minimum torque(per bolt).
Mt = $mu * FA * 1.1 * SQR(4 * Ae / PI) (16.12-81) = 0.2 * 3327.27 * 1.1 * SQR(4 * 36.6 / 3.14) = 4.9970$ Nm
Design anchor bolt load FBd
FBd = $MAX(FA, FB) (16.12.83) = MAX(3327.27, -117.78) = 3327.27$ N
Required Bolting Area
Aemin = $MAX(FB, 0) / fdAnchor = MAX(-117.78, 0) / 181.82 = 0.00$ mm²

Anchor Bolt Area Aemin=0 <= Ae=36.6[mm²]	0.0%	OK
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Spacing between anchor bolts
 $b8 = PI * (Dz_b + ea3 + 2 * b3) / n = 3.14 * (114 + 3 + 2 * 23) / 4 = 128.02$ mm
Design load on concrete FCd
FCd = $MAX(4 * M5 / DCR + F5, FA * n) = MAX(4 * 1697.62 / 120 + 512.77, 3327.27 * 4) = 13309.09$ N
Foundation Bearing Pressure on concrete below base ring
Sigc = $FCd / (PI * DCR * b1) (16.12-86) = 13309.09 / (3.14 * 120 * 70) = 0.5043$ N/mm²

Foundation Bearing Pressure(concrete) Sigc=0.5043 <= fcd/1.35=5.48[N/mm²] (16.12-86)	9.2%	OK
---	-------------	-----------

16.12.5.4.3 Checks for type 1 - Simple Bearing Plate

Required minimum analysis thickness of bearing plate (FB <= 0)
 $ea4min = b2 * SQR(3 * Sigc / f4) (16.12-90b) = 36.5 * SQR(3 * 0.5043 / 223.81) = 3.0011$ mm

Min.Thk.of Base Ring ea4min=3. <= eR=5[mm]	60.0%	OK
--	--------------	-----------

LOAD CASE NO: 2 - LC4 SHUT DOWN

Summation of Total Loads for Load Case : LC4 SHUT DOWN

Fz (Force in Vertical Direction)= == -0.3672 kN
Fx (Force in X-Direction)= == 0.0063 kN
Fy (Force in Y-Direction)= == 0.00 kN
Fs= $SQR(Fx^2 + Fy^2)$ (Tot.Force in Hor.Plane)= == 0.0063 kN
Moments at top of skirt at elevation of shell/skirt connection.
Mx (Moment around the X-Axis)= == -0.8814 kNmm
My (Moment around the Y-Axis)= == 2.2874 kNmm
Ml= $SQR(Mx^2 + My^2)$ (Moment in XY Plane)= == 2.4513 kNmm
Mt (Moment around the z-axis)= == 0.0235 kNmm
Moments at elevation at bottom of skirt.
Mx4 (Moment around the X-Axis)= == -0.8814 kNmm
My4 (Moment around the Y-Axis)= == 2.5403 kNmm
M4= $SQR(Mx4^2 + My4^2)$ (Moment in XY Plane)= == 2.6889 kNmm

Company Name -

Client :GÜVEN SOGUTMA

Vessel Tag No.:OS.C.130B.35.1

Visual Vessel Design by Hexagon PPM,Ver:20.0 Operator : Rev.:A

EN13445:2014 Issue 5:2018+A5 - EN13445; 16.12 - VERTICAL VESSELS WITH SKIRT

SK.1 Skirt 09 Feb. 2021 14:35 ConnID:S1.1

FF (Weight of Content of Vessel)= == 0.00 kN
dFg (Weight of Vessel Below Section 2-2)= (16.12-3) == 2,5707E-04 kN

16.12.6.1 Verification of Membrane Stresses

Fzp = Fz + 4 * M1 / Dz (16.12-1) = -367.2+4*2.45/114= -281.16 N
Fzq = Fz - 4 * M1 / Dz (16.12-2) = -367.2-4*2.45/114= -453.18 N

16.14.8 COMPRESSIVE STRESS LIMITS

K = 1.21 * E * ea / (Sige * D) (16.14-15)
=1.21*2.0607E05*5.39/(304*162.41)= 27.22
alfa = 0.83 / Sqr(1 + 0.005 * D / ea) (16.14-16)
=0.83/Sqr(1+0.005*162.41/5.39)= 0.7738
delta = (1 - 0.4123 / (alfa * K) ^ 0.6) / S (16.14-19)
=(1-0.4123/(0.7738*27.22)^0.6)/1.5= 0.6225
Maximum Allowable Compressive Stress
Sigcall = Sige * delta (16.14-20) =304*0.6225= 189.24 N/mm2

Stresses in section 1-1 (internal pressure)

Membrane Stress(Sect.1-1)
Sig1pm = (Fzp+dFg+FF)/(PI*DB*eB)+p*DB/(4*eB)
=(-281.16+0.2571+0)/(3.14*162.41*5.39)+0*162.41/(4*5.39)= -0.1021 N/mm2
Sig1qm = (Fzq+dFg+FF)/(PI*DB*eB)+p*DB/(4*eB)
=(-453.18+0.2571+0)/(3.14*162.41*5.39)+0*162.41/(4*5.39)= -0.1647 N/mm2

Compr.Stress(Pint)(Sect.1-1) Sig1pm=0.1021 <= Sigcall=189.24[N/mm2]	0.0%	OK
---	------	----

Compr.Stress(Pint)(Sect.1-1) Sig1qm=0.1647 <= Sigcall=189.24[N/mm2]	0.0%	OK
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Stresses in section 1-1 (external pressure)

Membrane Stress(Sect.1-1)
Sig1pmext = (Fzp+dFg+FF)/(PI*DB*eB)-Pext*DB/(4*eB)
=(-281.16+0.2571+0)/(3.14*162.41*5.39)-0*162.41/(4*5.39)= -0.1021 N/mm2
Sig1qmext = (Fzq+dFg+FF)/(PI*DB*eB)-Pext*DB/(4*eB)
=(-453.18+0.2571+0)/(3.14*162.41*5.39)-0*162.41/(4*5.39)= -0.1647 N/mm2

Compr.Stress(Pext)(Sect.1-1) Sig1pmext=0.1021 <= Sigcall=189.24[N/mm2]	0.0%	OK
--	------	----

Compr.Stress(Pext)(Sect.1-1) Sig1qmext=0.1647 <= Sigcall=189.24[N/mm2]	0.0%	OK
--	------	----

Membrane Stress(Sect.2-2)
Sig2pm = Sig2qm=(FF+dFg)/(PI*DB*eB)+p*DB/(4*eB)
=(0+0.2571)/(3.14*162.41*5.39)+0*162.41/(4*5.39)= 9,3477E-05 N/mm2

Membrane Stress(Sect.2-2) ABS(Sig2pm)=9.3477E-05 <= fB=202.67[N/mm2]	0.0%	OK
--	------	----

Membrane Stress(Sect.3-3)
Sig3pm = Fzp / (PI * Dz * ez) = -281.16/(3.14*114*3)= -0.2617 N/mm2
Sig3qm = Fzq / (PI * Dz * ez) = -453.18/(3.14*114*3)= -0.4218 N/mm2

Membrane Stress(Sect.3-3) ABS(Sig3pm)=0.2617 <= fz=78.33[N/mm2]	0.3%	OK
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Membrane Stress(Sect.3-3) ABS(Sig3qm)=0.4218 <= fz=78.33[N/mm2]	0.5%	OK
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Company Name -

Client :GÜVEN SOGUTMA

Vessel Tag No.:OS.C.130B.35.1

Visual Vessel Design by Hexagon PPM,Ver:20.0 Operator : Rev.:A

EN13445:2014 Issue 5:2018+A5 - EN13445; 16.12 - VERTICAL VESSELS WITH SKIRT

SK.1 Skirt 09 Feb. 2021 14:35 ConnID:S1.1

16.12.6.2 Bending Stresses

a) Structural Shape A - figure 16.12-1

Total Bending Moments at Verification Points p and q:

$$M_p = 0.5 * (D_z - DB) * F_{zp} \text{ (16.12-18)} = 0.5 * (114 - 162.41) * -281.16 = 6805.43 \text{ Nmm}$$

$$M_q = 0.5 * (D_z - DB) * F_{zq} \text{ (16.12-19)} = 0.5 * (114 - 162.41) * -453.18 = 10969.23 \text{ Nmm}$$

$$e_{1pm} = ((F_{zp} + dF_g + FF) / (\pi * DB) + p * DB / 4) / f_B \text{ (16.12-7)}$$

$$= ((-281.16 + 0.2571 + 0) / (3.14 * 162.41) + 0 * 162.41 / 4) / 202.67 = -0.0027 \text{ mm}$$

$$e_{1qm} = ((F_{zq} + dF_g + FF) / (\pi * DB) + p * DB / 4) / f_B \text{ (16.12-8)}$$

$$= ((-453.18 + 0.2571 + 0) / (3.14 * 162.41) + 0 * 162.41 / 4) / 202.67 = -0.0044 \text{ mm}$$

$$e_{2m} = ((dF_g + FF) / (\pi * DB) + p * DB / 4) / f_B \text{ (16.12-11)}$$

$$= ((0.2571 + 0) / (3.14 * 162.41) + 0 * 162.41 / 4) / 202.67 = 2.486E-06 \text{ mm}$$

$$e_{3pm} = (F_{zp} / (\pi * D_z)) / f_B \text{ (16.12-11)}$$

$$= (-281.16 / (3.14 * 114)) / 202.67 = -0.0039 \text{ mm}$$

$$e_{3qm} = (F_{zq} / (\pi * D_z)) / f_B \text{ (16.12-11)}$$

$$= (-453.18 / (3.14 * 114)) / 202.67 = -0.0062 \text{ mm}$$

Total Section Modulus of the Support Ring at Verification Point

$$W_p = \pi / 4 * ((D_z + e_z - DB - e_B) * h^2 + (2 * e_B^2 - e_{1pm}^2 - e_{2m}^2) * DB + 0.5 * (e_{2m}^2 - e_{3pm}^2) * D_z) \text{ (16.12-20)}$$

$$= 3.14 / 4 * ((114 + 3 - 162.41 - 5.39) * 0^2 + (2 * 5.39^2 - 0.0027^2 - 2.486E-06^2) * 162.41 + 0.5 * (2.486E-06^2 - 0.0039^2) * 114) = 7411.57 \text{ mm}^3$$

$$W_q = \pi / 4 * ((D_z + e_z - DB - e_B) * h^2 + (2 * e_B^2 - e_{1qm}^2 - e_{2m}^2) * DB + 0.5 * (e_{2m}^2 - e_{3qm}^2) * D_z) \text{ (16.12-21)}$$

$$= 3.14 / 4 * ((114 + 3 - 162.41 - 5.39) * 0^2 + (2 * 5.39^2 - 0.0044^2 - 2.486E-06^2) * 162.41 + 0.5 * (2.486E-06^2 - 0.0062^2) * 114) = 7411.56 \text{ mm}^3$$

16.12.6.3 Total Stresses and Strength Conditions

a) Structural Shape A

1) Location p ABS(M_p)/ $W_p=0.9182 \leq f_T=78.33$ [N/mm ²] (16.12-44)	1.1%	OK
2) Location q ABS(M_q)/ $W_q=1.48 \leq f_T=78.33$ [N/mm ²] (16.12-45)	1.8%	OK

16.12.7 Verification of Skirt (Section 4-4)

Note: Skirt is pierced and has a minimum section modulus at $z = 0$ mm

Verification of Skirt at Bottom(no openings) at Elevation $z = -60$ mm

Additional Moment due to Displacement of Neutral Axis

$$dM_4 = \epsilon_s * F_4 = 0 * 367.17 = 0.00 \text{ kNmm}$$

Membrane Stress(Sect.4-4/ $z=-60$)

$$\sigma_{4pm} = (M_{44} + dM_4) / W_4 - F_4 / A_4 = (2688.88 + 0) / 29856.61 - 367.17 / 1074.43 = -0.2517 \text{ N/mm}^2$$

$$\sigma_{4qm} = -1 * (M_{44} + dM_4) / W_4 - F_4 / A_4 = -1 * (2688.88 + 0) / 29856.61 - 367.17 / 1074.43 = -0.4318 \text{ N/mm}^2$$

Membrane Stress(Sect.4-4/ $z=-60$) $\sigma_{4pm}=0.2517 \leq f_z=78.33$ [N/mm ²]	0.3%	OK
Membrane Stress(Sect.4-4/ $z=-60$) $\sigma_{4qm}=0.4318 \leq f_z=78.33$ [N/mm ²]	0.5%	OK

Compressive Stresses in Skirt(Sect.4-4/ $z=-60$)

$$\sigma_{4c} = -F_4 / A_4 = -367.17 / 1074.43 = -0.3417 \text{ N/mm}^2$$

16.14.8 COMPRESSIVE STRESS LIMITS

$$K = 1.21 * E * e_a / (\sigma_{ige} * D) \text{ (16.14-15)} = 1.21 * 206070 * 3 / (235 * 114) = 27.92$$

$$\alpha = 0.83 / \sqrt{1 + 0.005 * D / e_a} \text{ (16.14-16)} = 0.83 / \sqrt{1 + 0.005 * 114 / 3} = 0.7609$$

$$\Delta = (1 - 0.4123 / (\alpha * K)^{0.6}) / S \text{ (16.14-19)} = (1 - 0.4123 / (0.7609 * 27.92)^{0.6}) / 1.5 = 0.6227$$

$$\sigma_{call} = \sigma_{ige} * \Delta \text{ (16.14-20)} = 235 * 0.6227 = 146.34 \text{ N/mm}^2$$

Company Name -

Client :GÜVEN SOGUTMA Vessel Tag No.:OS.C.130B.35.1

Visual Vessel Design by Hexagon PPM,Ver:20.0 Operator : Rev.:A

EN13445:2014 Issue 5:2018+A5 - EN13445; 16.12 - VERTICAL VESSELS WITH SKIRT

SK.1 Skirt 09 Feb. 2021 14:35 ConnID:S1.1

Compr.Stress Limits (4-4/z=-60) Sig4c=0.3417 <= Sigcall=146.34[N/mm2]	0.2%	OK
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Check of Skirt Stability/Weakening due to Openings in Skirt

Maximum Compressive Force at Section 4-4

$$F_{c4max} = \pi * D_4 * e_z * \text{Sigcall} \quad (16.14-2) = 3.14 * 114 * 3 * 146.34 = 157.23 \text{ kN}$$

Maximum Bending Moment at Section 4-4

$$M_{4max} = \pi / 4 * D_4^2 * e_z * \text{Sigcall} \quad (16.14-3) \\ = 3.14 / 4 * 114^2 * 3 * 146.34 = 4.4812 \text{ kNm}$$

Stability of skirt(Sect.4-4/z=-60)

$$\text{StabilityFactor} = F_4 / (\text{Psi1} * F_{c4max}) + (\text{ABS}(M_{44}) + F_4 * \text{eps}) / (\text{Psi2} * M_{4max}) \quad (16.12-70) \\ = 367.17 / (1 * 1.5723E05) + (\text{ABS}(2688.88) + 367.17 * 0) / (1 * 4.4812E06) = 0.0029$$

Stability/Openings in skirt(Sect.4-4/z=-60) StabilityFactor=0.0029 <= 1=1	0.2%	OK
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Verification of Skirt at Top(no openings) at Elevation z = -20 mm

Additional Moment due to Displacement of Neutral Axis

$$dM_4 = \text{eps} * F_4 = 0 * 367.17 = 0.00 \text{ kNm}$$

Membrane Stress(Sect.4-4/z=-20)

$$\text{Sig4pm} = (M_{44} + dM_4) / W_4 - F_4 / A_4 \\ = (2451.32 + 0) / 29856.61 - 367.17 / 1074.43 = -0.2596 \text{ N/mm}^2$$

Sig4qm = - 1 * (M44 + dM4) / W4 - F4 / A4

$$= -1 * (2451.32 + 0) / 29856.61 - 367.17 / 1074.43 = -0.4238 \text{ N/mm}^2$$

Membrane Stress(Sect.4-4/z=-20) Sig4pm=0.2596 <= fz=78.33[N/mm2]	0.3%	OK
--	-------------	-----------

Membrane Stress(Sect.4-4/z=-20) Sig4qm=0.4238 <= fz=78.33[N/mm2]	0.5%	OK
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Compressive Stresses in Skirt(Sect.4-4/z=-20)

$$\text{Sig4c} = - F_4 / A_4 = -367.17 / 1074.43 = -0.3417 \text{ N/mm}^2$$

16.14.8 COMPRESSIVE STRESS LIMITS

$$K = 1.21 * E * e_a / (\text{Sige} * D) \quad (16.14-15) \\ = 1.21 * 206070 * 3 / (235 * 114) = 27.92$$

$$\text{alfa} = 0.83 / \text{Sqr}(1 + 0.005 * D / e_a) \quad (16.14-16) \\ = 0.83 / \text{Sqr}(1 + 0.005 * 114 / 3) = 0.7609$$

$$\text{delta} = (1 - 0.4123 / (\text{alfa} * K)^{0.6}) / S \quad (16.14-19) \\ = (1 - 0.4123 / (0.7609 * 27.92)^{0.6}) / 1.5 = 0.6227$$

$$\text{Maximum Allowable Compressive Stress} \\ \text{Sigcall} = \text{Sige} * \text{delta} \quad (16.14-20) = 235 * 0.6227 = 146.34 \text{ N/mm}^2$$

$$\text{Sigcall} = 235 * 0.6227 = 146.34 \text{ N/mm}^2$$

Compr.Stress Limits (4-4/z=-20) Sig4c=0.3417 <= Sigcall=146.34[N/mm2]	0.2%	OK
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Check of Skirt Stability/Weakening due to Openings in Skirt

Maximum Compressive Force at Section 4-4

$$F_{c4max} = \pi * D_4 * e_z * \text{Sigcall} \quad (16.14-2) = 3.14 * 114 * 3 * 146.34 = 157.23 \text{ kN}$$

Maximum Bending Moment at Section 4-4

$$M_{4max} = \pi / 4 * D_4^2 * e_z * \text{Sigcall} \quad (16.14-3) \\ = 3.14 / 4 * 114^2 * 3 * 146.34 = 4.4812 \text{ kNm}$$

Stability of skirt(Sect.4-4/z=-20)

$$\text{StabilityFactor} = F_4 / (\text{Psi1} * F_{c4max}) + (\text{ABS}(M_{44}) + F_4 * \text{eps}) / (\text{Psi2} * M_{4max}) \quad (16.12-70) \\ = 367.17 / (1 * 1.5723E05) + (\text{ABS}(2451.32) + 367.17 * 0) / (1 * 4.4812E06) = 0.0029$$

Stability/Openings in skirt(Sect.4-4/z=-20) StabilityFactor=0.0029 <= 1=1	0.2%	OK
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Company Name -

Client :GÜVEN SOGUTMA

Vessel Tag No.:OS.C.130B.35.1

Visual Vessel Design by Hexagon PPM,Ver:20.0 Operator : Rev.:A

EN13445:2014 Issue 5:2018+A5 - EN13445; 16.12 - VERTICAL VESSELS WITH SKIRT

SK.1 Skirt 09 Feb. 2021 14:35 ConnID:S1.1

SKIRT BASE DESIGN

Wsk = $F4 / (PI * Dz_b) = 367.17 / (3.14 * 114) = 1.0252$ N/mm
 Msk = $4 * M4 / (PI * Dz_b^2) = 4 * 2.69 / (3.14 * 114^2) = 0.2634$ N/mm
 Outer radial width of bearing plate/base plate
 $b2 = (dob - (Dz_b + ez)) / 2 = (190 - (114 + 3)) / 2 = 36.50$ mm

16.12.5 Anchor Bolt Design

Bolt Circle Diameter
 $d = Dz_b + ea3 + 2 * b3 = 114 + 3 + 2 * 23 = 163.00$ mm
 Mean Diameter of Bearing Ring/Base Plate
 $DCR = dob - b1 = 190 - 70 = 120.00$ mm
 Inside Diameter of Bearing Ring/Base Plate
 $D3 = dob - 2 * b1 = 190 - 2 * 70 = 50.00$ mm
 Pre-tensioning of bolts FA
 $FA = Phi * Ae * fdAnchor (16.12.80) = 0.5 * 36.6 * 181.82 = 3327.27$ N
 Maximum anchor bolt force FB
 $FB = (4 * M5 / d - F5) / n (16.12.77) = (4 * 2688.88 / 163 - 367.17) / 4 = -75.30$ N
 Required minimum torque(per bolt).
 $Mt = \mu * FA * 1.1 * SQR(4 * Ae / PI) (16.12-81) = 0.2 * 3327.27 * 1.1 * SQR(4 * 36.6 / 3.14) = 4.9970$ Nm
 Design anchor bolt load FBd
 $FBd = MAX(FA, FB) (16.12.83) = MAX(3327.27, -75.3) = 3327.27$ N
 Required Bolting Area
 $Aemin = MAX(FB, 0) / fdAnchor = MAX(-75.3, 0) / 181.82 = 0.00$ mm²

Anchor Bolt Area Aemin=0 <= Ae=36.6[mm²] **0.0%** **OK**

Spacing between anchor bolts
 $b8 = PI * (Dz_b + ea3 + 2 * b3) / n = 3.14 * (114 + 3 + 2 * 23) / 4 = 128.02$ mm
 Design load on concrete FCd
 $FCd = MAX(4 * M5 / DCR + F5, FA * n) = MAX(4 * 2688.88 / 120 + 367.17, 3327.27 * 4) = 13309.09$ N
 Foundation Bearing Pressure on concrete below base ring
 $Sigc = FCd / (PI * DCR * b1) (16.12-86) = 13309.09 / (3.14 * 120 * 70) = 0.5043$ N/mm²

Foundation Bearing Pressure(concrete) Sigc=0.5043 <= fcd/1.35=5.48[N/mm²] (16.12-86) **9.2%** **OK**

16.12.5.4.3 Checks for type 1 - Simple Bearing Plate

Required minimum analysis thickness of bearing plate (FB <= 0)
 $ea4min = b2 * SQR(3 * Sigc / f4) (16.12-90b) = 36.5 * SQR(3 * 0.5043 / 142.67) = 3.7588$ mm

Min.Thk.of Base Ring ea4min=3.76 <= eR=5[mm] **75.1%** **OK**

LOAD CASE NO: 3 - LC5 INSTALLATION

Summation of Total Loads for Load Case : LC5 INSTALLATION

Fz (Force in Vertical Direction)= == -0.3672 kN
 Fx (Force in X-Direction)= == 0.0040 kN
 Fy (Force in Y-Direction)= == 0.00 kN
 Fs=SQR(Fx^2 + Fy^2)(Tot.Force in Hor.Plane)= == 0.0040 kN
 Moments at top of skirt at elevation of shell/skirt connection.
 Mx (Moment around the X-Axis)= == -0.8814 kNm
 My (Moment around the Y-Axis)= == 1.5078 kNm
 M1=SQR(Mx^2+My^2)(Moment in XY Plane)= == 1.7465 kNm
 Mt (Moment around the z-axis)= == 0.0149 kNm
 Moments at elevation at bottom of skirt.
 Mx4 (Moment around the X-Axis)= == -0.8814 kNm
 My4 (Moment around the Y-Axis)= == 1.6687 kNm
 M4=SQR(Mx4^2+My4^2)(Moment in XY Plane)= == 1.8872 kNm
 FF (Weight of Content of Vessel)= == 0.00 kN

Company Name -

Client :GÜVEN SOGUTMA

Vessel Tag No.:OS.C.130B.35.1

Visual Vessel Design by Hexagon PPM,Ver:20.0 Operator : Rev.:A

EN13445:2014 Issue 5:2018+A5 - EN13445; 16.12 - VERTICAL VESSELS WITH SKIRT

SK.1 Skirt 09 Feb. 2021 14:35 ConnID:S1.1

dFg (Weight of Vessel Below Section 2-2)= (16.12-3) == 2,5707E-04 kN

16.12.6.1 Verification of Membrane Stresses

Fzp = Fz + 4 * M1 / Dz (16.12-1) = -367.2+4*1.75/114= -305.89 N

Fzq = Fz - 4 * M1 / Dz (16.12-2) = -367.2-4*1.75/114= -428.45 N

16.14.8 COMPRESSIVE STRESS LIMITS

K = 1.21 * E * ea / (Sige * D) (16.14-15)

=1.21*2.1177E05*5.39/(355*162.41)= 23.96

alfa = 0.83 / Sqr(1 + 0.005 * D / ea) (16.14-16)

=0.83/Sqr(1+0.005*162.41/5.39)= 0.7738

delta = (1 - 0.4123 / (alfa * K) ^ 0.6) / S (16.14-19)

=(1-0.4123/(0.7738*23.96)^0.6)/1.5= 0.6190

Maximum Allowable Compressive Stress

Sigcall = Sige * delta (16.14-20) =355*0.619= 219.74 N/mm2

Stresses in section 1-1 (internal pressure)

Membrane Stress(Sect.1-1)

Sig1pm = (Fzp+dFg+FF)/(PI*DB*eB)+p*DB/(4*eB)
 =(-305.89+0.2571+0)/(3.14*162.41*5.39)+0*162.41/(4*5.39)= -0.1111 N/mm2

Sig1qm = (Fzq+dFg+FF)/(PI*DB*eB)+p*DB/(4*eB)
 =(-428.45+0.2571+0)/(3.14*162.41*5.39)+0*162.41/(4*5.39)= -0.1557 N/mm2

Compr.Stress(Pint)(Sect.1-1) Sig1pm=0.1111 <= Sigcall=219.74[N/mm2]	0.0%	OK
---	------	----

Compr.Stress(Pint)(Sect.1-1) Sig1qm=0.1557 <= Sigcall=219.74[N/mm2]	0.0%	OK
---	------	----

Stresses in section 1-1 (external pressure)

Membrane Stress(Sect.1-1)

Sig1pmext = (Fzp+dFg+FF)/(PI*DB*eB)-Pext*DB/(4*eB)
 =(-305.89+0.2571+0)/(3.14*162.41*5.39)-0*162.41/(4*5.39)= -0.1111 N/mm2

Sig1qmext = (Fzq+dFg+FF)/(PI*DB*eB)-Pext*DB/(4*eB)
 =(-428.45+0.2571+0)/(3.14*162.41*5.39)-0*162.41/(4*5.39)= -0.1557 N/mm2

Compr.Stress(Pext)(Sect.1-1) Sig1pmext=0.1111 <= Sigcall=219.74[N/mm2]	0.0%	OK
--	------	----

Compr.Stress(Pext)(Sect.1-1) Sig1qmext=0.1557 <= Sigcall=219.74[N/mm2]	0.0%	OK
--	------	----

Membrane Stress(Sect.2-2)

Sig2pm = Sig2qm=(FF+dFg)/(PI*DB*eB)+p*DB/(4*eB)
 =(0+0.2571)/(3.14*162.41*5.39)+0*162.41/(4*5.39)= 9,3477E-05 N/mm2

Membrane Stress(Sect.2-2) ABS(Sig2pm)=9.3477E-05 <= fB=204.17[N/mm2]	0.0%	OK
--	------	----

Membrane Stress(Sect.3-3)

Sig3pm = Fzp / (PI * Dz * ez) = -305.89/(3.14*114*3)= -0.2847 N/mm2

Sig3qm = Fzq / (PI * Dz * ez) = -428.45/(3.14*114*3)= -0.3988 N/mm2

Membrane Stress(Sect.3-3) ABS(Sig3pm)=0.2847 <= fz=78.33[N/mm2]	0.3%	OK
---	------	----

Membrane Stress(Sect.3-3) ABS(Sig3qm)=0.3988 <= fz=78.33[N/mm2]	0.5%	OK
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Company Name -

Client :GÜVEN SOGUTMA

Vessel Tag No.:OS.C.130B.35.1

Visual Vessel Design by Hexagon PPM,Ver:20.0 Operator : Rev.:A

EN13445:2014 Issue 5:2018+A5 - EN13445; 16.12 - VERTICAL VESSELS WITH SKIRT

SK.1 Skirt 09 Feb. 2021 14:35 ConnID:S1.1

16.12.6.2 Bending Stresses

a) Structural Shape A - figure 16.12-1

Total Bending Moments at Verification Points p and q:

$$M_p = 0.5 * (D_z - DB) * F_{zp} \text{ (16.12-18)} = 0.5 * (114 - 162.41) * -305.89 = 7404.03 \text{ Nmm}$$

$$M_q = 0.5 * (D_z - DB) * F_{zq} \text{ (16.12-19)} = 0.5 * (114 - 162.41) * -428.45 = 10370.63 \text{ Nmm}$$

$$e_{1pm} = ((F_{zp} + dF_g + FF) / (\pi * DB) + p * DB / 4) / f_B \text{ (16.12-7)}$$

$$= ((-305.89 + 0.2571 + 0) / (3.14 * 162.41) + 0 * 162.41 / 4) / 204.17 = -0.0029 \text{ mm}$$

$$e_{1qm} = ((F_{zq} + dF_g + FF) / (\pi * DB) + p * DB / 4) / f_B \text{ (16.12-8)}$$

$$= ((-428.45 + 0.2571 + 0) / (3.14 * 162.41) + 0 * 162.41 / 4) / 204.17 = -0.0041 \text{ mm}$$

$$e_{2m} = ((dF_g + FF) / (\pi * DB) + p * DB / 4) / f_B \text{ (16.12-11)}$$

$$= ((0.2571 + 0) / (3.14 * 162.41) + 0 * 162.41 / 4) / 204.17 = 2.4678E-06 \text{ mm}$$

$$e_{3pm} = (F_{zp} / (\pi * D_z)) / f_B \text{ (16.12-11)}$$

$$= (-305.89 / (3.14 * 114)) / 204.17 = -0.0042 \text{ mm}$$

$$e_{3qm} = (F_{zq} / (\pi * D_z)) / f_B \text{ (16.12-11)}$$

$$= (-428.45 / (3.14 * 114)) / 204.17 = -0.0059 \text{ mm}$$

Total Section Modulus of the Support Ring at Verification Point

$$W_p = \pi / 4 * ((D_z + e_z - DB - e_B) * h^2 + (2 * e_B^2 - e_{1pm}^2 - e_{2m}^2) * DB + 0.5 * (e_{2m}^2 - e_{3pm}^2) * D_z) \text{ (16.12-20)}$$

$$= 3.14 / 4 * ((114 + 3 - 162.41 - 5.39) * 0^2 + (2 * 5.39^2 - 0.0029^2 - 2.4678E-06^2) * 162.41 + 0.5 * (2.4678E-06^2 - 0.0042^2) * 114) = 7411.57 \text{ mm}^3$$

$$W_q = \pi / 4 * ((D_z + e_z - DB - e_B) * h^2 + (2 * e_B^2 - e_{1qm}^2 - e_{2m}^2) * DB + 0.5 * (e_{2m}^2 - e_{3qm}^2) * D_z) \text{ (16.12-21)}$$

$$= 3.14 / 4 * ((114 + 3 - 162.41 - 5.39) * 0^2 + (2 * 5.39^2 - 0.0041^2 - 2.4678E-06^2) * 162.41 + 0.5 * (2.4678E-06^2 - 0.0059^2) * 114) = 7411.56 \text{ mm}^3$$

16.12.6.3 Total Stresses and Strength Conditions

a) Structural Shape A

1) Location p ABS(M_p)/ $W_p=0.999 \leq f_T=78.33$ [N/mm ²] (16.12-44)	1.2%	OK
2) Location q ABS(M_q)/ $W_q=1.4 \leq f_T=78.33$ [N/mm ²] (16.12-45)	1.7%	OK

16.12.7 Verification of Skirt (Section 4-4)

Note: Skirt is pierced and has a minimum section modulus at z = 0mm

Verification of Skirt at Bottom(no openings) at Elevation z = -60 mm

Additional Moment due to Displacement of Neutral Axis

$$dM_4 = \epsilon_p * F_4 = 0 * 367.17 = 0.00 \text{ kNm}$$

Membrane Stress(Sect.4-4/z=-60)

$$\sigma_{4pm} = (M_{44} + dM_4) / W_4 - F_4 / A_4 = (1887.22 + 0) / 29856.61 - 367.17 / 1074.43 = -0.2785 \text{ N/mm}^2$$

$\sigma_{4qm} = -1 * (M_{44} + dM_4) / W_4 - F_4 / A_4$

$$= -1 * (1887.22 + 0) / 29856.61 - 367.17 / 1074.43 = -0.4049 \text{ N/mm}^2$$

Membrane Stress(Sect.4-4/z=-60) $\sigma_{4pm}=0.2785 \leq f_z=78.33$ [N/mm ²]	0.3%	OK
Membrane Stress(Sect.4-4/z=-60) $\sigma_{4qm}=0.4049 \leq f_z=78.33$ [N/mm ²]	0.5%	OK

Compressive Stresses in Skirt(Sect.4-4/z=-60)

$$\sigma_{4c} = -F_4 / A_4 = -367.17 / 1074.43 = -0.3417 \text{ N/mm}^2$$

16.14.8 COMPRESSIVE STRESS LIMITS

$$K = 1.21 * E * e_a / (\sigma_{ige} * D) \text{ (16.14-15)}$$

$$= 1.21 * 206070 * 3 / (235 * 114) = 27.92$$

$$\alpha = 0.83 / \sqrt{1 + 0.005 * D / e_a} \text{ (16.14-16)}$$

$$= 0.83 / \sqrt{1 + 0.005 * 114 / 3} = 0.7609$$

$$\delta = (1 - 0.4123 / (\alpha * K) ^{0.6}) / S \text{ (16.14-19)}$$

$$= (1 - 0.4123 / (0.7609 * 27.92) ^{0.6}) / 1.5 = 0.6227$$

Maximum Allowable Compressive Stress

$$\sigma_{call} = \sigma_{ige} * \delta \text{ (16.14-20)} = 235 * 0.6227 = 146.34 \text{ N/mm}^2$$

Company Name -

Client :GÜVEN SOGUTMA Vessel Tag No.:OS.C.130B.35.1

Visual Vessel Design by Hexagon PPM,Ver:20.0 Operator : Rev.:A

EN13445:2014 Issue 5:2018+A5 - EN13445; 16.12 - VERTICAL VESSELS WITH SKIRT

SK.1 Skirt 09 Feb. 2021 14:35 ConnID:S1.1

Compr.Stress Limits (4-4/z=-60) Sig4c=0.3417 <= Sigcall=146.34[N/mm2]	0.2%	OK
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Check of Skirt Stability/Weakening due to Openings in Skirt

Maximum Compressive Force at Section 4-4

$$F_{c4max} = \pi * D_4 * e_z * \text{Sigcall} \quad (16.14-2) = 3.14 * 114 * 3 * 146.34 = 157.23 \text{ kN}$$

Maximum Bending Moment at Section 4-4

$$M_{4max} = \pi / 4 * D_4^2 * e_z * \text{Sigcall} \quad (16.14-3) \\ = 3.14 / 4 * 114^2 * 3 * 146.34 = 4.4812 \text{ kNm}$$

Stability of skirt(Sect.4-4/z=-60)

$$\text{StabilityFactor} = F_4 / (\text{Psi1} * F_{c4max}) + (\text{ABS}(M_{44}) + F_4 * \text{eps}) / (\text{Psi2} * M_{4max}) \quad (16.12-70) \\ = 367.17 / (1 * 1.5723E05) + (\text{ABS}(1887.22) + 367.17 * 0) / (1 * 4.4812E06) = 0.0028$$

Stability/Openings in skirt(Sect.4-4/z=-60) StabilityFactor=0.0028 <= 1=1	0.2%	OK
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Verification of Skirt at Top(no openings) at Elevation z = -20 mm

Additional Moment due to Displacement of Neutral Axis

$$dM_4 = \text{eps} * F_4 = 0 * 367.17 = 0.00 \text{ kNm}$$

Membrane Stress(Sect.4-4/z=-20)

$$\text{Sig4pm} = (M_{44} + dM_4) / W_4 - F_4 / A_4 \\ = (1746.51 + 0) / 29856.61 - 367.17 / 1074.43 = -0.2832 \text{ N/mm}^2$$

Sig4qm = - 1 * (M44 + dM4) / W4 - F4 / A4

$$= -1 * (1746.51 + 0) / 29856.61 - 367.17 / 1074.43 = -0.4002 \text{ N/mm}^2$$

Membrane Stress(Sect.4-4/z=-20) Sig4pm=0.2832 <= fz=78.33[N/mm2]	0.3%	OK
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Membrane Stress(Sect.4-4/z=-20) Sig4qm=0.4002 <= fz=78.33[N/mm2]	0.5%	OK
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Compressive Stresses in Skirt(Sect.4-4/z=-20)

$$\text{Sig4c} = - F_4 / A_4 = -367.17 / 1074.43 = -0.3417 \text{ N/mm}^2$$

16.14.8 COMPRESSIVE STRESS LIMITS

$$K = 1.21 * E * e_a / (\text{Sige} * D) \quad (16.14-15) \\ = 1.21 * 206070 * 3 / (235 * 114) = 27.92$$

$$\text{alfa} = 0.83 / \text{Sqr}(1 + 0.005 * D / e_a) \quad (16.14-16) \\ = 0.83 / \text{Sqr}(1 + 0.005 * 114 / 3) = 0.7609$$

$$\text{delta} = (1 - 0.4123 / (\text{alfa} * K)^{0.6}) / S \quad (16.14-19) \\ = (1 - 0.4123 / (0.7609 * 27.92)^{0.6}) / 1.5 = 0.6227$$

$$\text{Maximum Allowable Compressive Stress} \\ \text{Sigcall} = \text{Sige} * \text{delta} \quad (16.14-20) = 235 * 0.6227 = 146.34 \text{ N/mm}^2$$

$$\text{Sigcall} = 235 * 0.6227 = 146.34 \text{ N/mm}^2$$

Compr.Stress Limits (4-4/z=-20) Sig4c=0.3417 <= Sigcall=146.34[N/mm2]	0.2%	OK
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Check of Skirt Stability/Weakening due to Openings in Skirt

Maximum Compressive Force at Section 4-4

$$F_{c4max} = \pi * D_4 * e_z * \text{Sigcall} \quad (16.14-2) = 3.14 * 114 * 3 * 146.34 = 157.23 \text{ kN}$$

Maximum Bending Moment at Section 4-4

$$M_{4max} = \pi / 4 * D_4^2 * e_z * \text{Sigcall} \quad (16.14-3) \\ = 3.14 / 4 * 114^2 * 3 * 146.34 = 4.4812 \text{ kNm}$$

Stability of skirt(Sect.4-4/z=-20)

$$\text{StabilityFactor} = F_4 / (\text{Psi1} * F_{c4max}) + (\text{ABS}(M_{44}) + F_4 * \text{eps}) / (\text{Psi2} * M_{4max}) \quad (16.12-70) \\ = 367.17 / (1 * 1.5723E05) + (\text{ABS}(1746.51) + 367.17 * 0) / (1 * 4.4812E06) = 0.0027$$

Stability/Openings in skirt(Sect.4-4/z=-20) StabilityFactor=0.0027 <= 1=1	0.2%	OK
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Company Name -

Client :GÜVEN SOGUTMA

Vessel Tag No.:OS.C.130B.35.1

Visual Vessel Design by Hexagon PPM,Ver:20.0 Operator : Rev.:A

EN13445:2014 Issue 5:2018+A5 - EN13445; 16.12 - VERTICAL VESSELS WITH SKIRT

SK.1 Skirt 09 Feb. 2021 14:35 ConnID:S1.1

SKIRT BASE DESIGN

Wsk = $F4 / (PI * Dz_b) = 367.17 / (3.14 * 114) = 1.0252$ N/mm
Msk = $4 * M4 / (PI * Dz_b^2) = 4 * 1.89 / (3.14 * 114^2) = 0.1849$ N/mm
Outer radial width of bearing plate/base plate
 $b2 = (dob - (Dz_b + ez)) / 2 = (190 - (114 + 3)) / 2 = 36.50$ mm

16.12.5 Anchor Bolt Design

Bolt Circle Diameter
 $d = Dz_b + ea3 + 2 * b3 = 114 + 3 + 2 * 23 = 163.00$ mm
Mean Diameter of Bearing Ring/Base Plate
DCR = $dob - b1 = 190 - 70 = 120.00$ mm
Inside Diameter of Bearing Ring/Base Plate
 $D3 = dob - 2 * b1 = 190 - 2 * 70 = 50.00$ mm
Pre-tensioning of bolts FA
 $FA = Phi * Ae * fdAnchor (16.12.80) = 0.5 * 36.6 * 181.82 = 3327.27$ N
Maximum anchor bolt force FB
 $FB = (4 * M5 / d - F5) / n (16.12.77) = (4 * 1887.22 / 163 - 367.17) / 4 = -80.21$ N
Required minimum torque(per bolt).
 $Mt = \mu * FA * 1.1 * SQR(4 * Ae / PI) (16.12-81) = 0.2 * 3327.27 * 1.1 * SQR(4 * 36.6 / 3.14) = 4.9970$ Nm
Design anchor bolt load FBd
 $FBd = MAX(FA, FB) (16.12.83) = MAX(3327.27, -80.21) = 3327.27$ N
Required Bolting Area
 $Aemin = MAX(FB, 0) / fdAnchor = MAX(-80.21, 0) / 181.82 = 0.00$ mm²

Anchor Bolt Area Aemin=0 <= Ae=36.6[mm²] **0.0%** **OK**

Spacing between anchor bolts
 $b8 = PI * (Dz_b + ea3 + 2 * b3) / n = 3.14 * (114 + 3 + 2 * 23) / 4 = 128.02$ mm
Design load on concrete FCd
 $FCd = MAX(4 * M5 / DCR + F5, FA * n) = MAX(4 * 1887.22 / 120 + 367.17, 3327.27 * 4) = 13309.09$ N
Foundation Bearing Pressure on concrete below base ring
 $Sigc = FCd / (PI * DCR * b1) (16.12-86) = 13309.09 / (3.14 * 120 * 70) = 0.5043$ N/mm²

Foundation Bearing Pressure(concrete) Sigc=0.5043 <= fcd/1.35=5.48[N/mm²] (16.12-86) **9.2%** **OK**

16.12.5.4.3 Checks for type 1 - Simple Bearing Plate

Required minimum analysis thickness of bearing plate (FB <= 0)
 $ea4min = b2 * SQR(3 * Sigc / f4) (16.12-90b) = 36.5 * SQR(3 * 0.5043 / 150) = 3.6658$ mm

Min.Thk.of Base Ring ea4min=3.67 <= eR=5[mm] **73.3%** **OK**

LOAD CASE NO: 4 - LC1&2&3 OPER.WIND

Summation of Total Loads for Load Case : LC1&2&3 OPER.WIND

Fz (Force in Vertical Direction)= == -0.4335 kN
Fx (Force in X-Direction)= == 0.0063 kN
Fy (Force in Y-Direction)= == 0.00 kN
Fs= $SQR(Fx^2 + Fy^2)$ (Tot.Force in Hor.Plane)= == 0.0063 kN
Moments at top of skirt at elevation of shell/skirt connection.
Mx (Moment around the X-Axis)= == -0.8814 kNm
My (Moment around the Y-Axis)= == 2.2874 kNm
M1= $SQR(Mx^2 + My^2)$ (Moment in XY Plane)= == 2.4513 kNm
Mt (Moment around the z-axis)= == 0.0235 kNm
Moments at elevation at bottom of skirt.
Mx4 (Moment around the X-Axis)= == -0.8814 kNm
My4 (Moment around the Y-Axis)= == 2.5403 kNm
M4= $SQR(Mx4^2 + My4^2)$ (Moment in XY Plane)= == 2.6889 kNm
FF (Weight of Content of Vessel)= == 0.0663 kN

Company Name -

Client :GÜVEN SOGUTMA

Vessel Tag No.:OS.C.130B.35.1

Visual Vessel Design by Hexagon PPM,Ver:20.0 Operator : Rev.:A

EN13445:2014 Issue 5:2018+A5 - EN13445; 16.12 - VERTICAL VESSELS WITH SKIRT

SK.1 Skirt 09 Feb. 2021 14:35 ConnID:S1.1

dFg (Weight of Vessel Below Section 2-2)= (16.12-3) == 2,5708E-04 kN

16.12.6.1 Verification of Membrane Stresses

Fzp = Fz + 4 * M1 / Dz (16.12-1) = -433.5+4*2.45/114= -347.46 N

Fzq = Fz - 4 * M1 / Dz (16.12-2) = -433.5-4*2.45/114= -519.49 N

16.14.8 COMPRESSIVE STRESS LIMITS

K = 1.21 * E * ea / (Sige * D) (16.14-15)

=1.21*2.0607E05*5.39/(304*162.41)= 27.22

alfa = 0.83 / Sqr(1 + 0.005 * D / ea) (16.14-16)

=0.83/Sqr(1+0.005*162.41/5.39)= 0.7738

delta = (1 - 0.4123 / (alfa * K) ^ 0.6) / S (16.14-19)

=(1-0.4123/(0.7738*27.22)^0.6)/1.5= 0.6225

Maximum Allowable Compressive Stress

Sigcall = Sige * delta (16.14-20) =304*0.6225= 189.24 N/mm2

Stresses in section 1-1 (internal pressure)

Membrane Stress(Sect.1-1)

Sig1pm = (Fzp+dFg+FF)/(PI*DB*eB)+p*DB/(4*eB)
 =(-347.46+0.2571+66.31)/(3.14*162.41*5.39)+11.7*162.41/(4*5.39)= 88.03 N/mm2

Sig1qm = (Fzq+dFg+FF)/(PI*DB*eB)+p*DB/(4*eB)
 =(-519.49+0.2571+66.31)/(3.14*162.41*5.39)+11.7*162.41/(4*5.39)= 87.97 N/mm2

Membrane Stress(Pint)(Sect.1-1) Sig1pm=88.03 <= fB=202.67[N/mm2]	43.4%	OK
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Membrane Stress(Pint)(Sect.1-1) ABS(Sig1qm)=87.97 <= fB=202.67[N/mm2]	43.4%	OK
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Stresses in section 1-1 (external pressure)

Membrane Stress(Sect.1-1)

Sig1pmext = (Fzp+dFg+FF)/(PI*DB*eB)-Pext*DB/(4*eB)
 =(-347.46+0.2571+66.31)/(3.14*162.41*5.39)-0*162.41/(4*5.39)= -0.1021 N/mm2

Sig1qmext = (Fzq+dFg+FF)/(PI*DB*eB)-Pext*DB/(4*eB)
 =(-519.49+0.2571+66.31)/(3.14*162.41*5.39)-0*162.41/(4*5.39)= -0.1647 N/mm2

Compr.Stress(Pext)(Sect.1-1) Sig1pmext=0.1021 <= Sigcall=189.24[N/mm2]	0.0%	OK
--	------	----

Compr.Stress(Pext)(Sect.1-1) Sig1qmext=0.1647 <= Sigcall=189.24[N/mm2]	0.0%	OK
--	------	----

Membrane Stress(Sect.2-2)

Sig2pm = Sig2qm=(FF+dFg)/(PI*DB*eB)+p*DB/(4*eB)
 =(66.31+0.2571)/(3.14*162.41*5.39)+11.7*162.41/(4*5.39)= 88.16 N/mm2

Membrane Stress(Sect.2-2) ABS(Sig2pm)=88.16 <= fB=202.67[N/mm2]	43.4%	OK
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Membrane Stress(Sect.3-3)

Sig3pm = Fzp / (PI * Dz * ez) = -347.46/(3.14*114*3)= -0.3234 N/mm2

Sig3qm = Fzq / (PI * Dz * ez) = -519.49/(3.14*114*3)= -0.4835 N/mm2

Membrane Stress(Sect.3-3) ABS(Sig3pm)=0.3234 <= fz=78.33[N/mm2]	0.4%	OK
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Membrane Stress(Sect.3-3) ABS(Sig3qm)=0.4835 <= fz=78.33[N/mm2]	0.6%	OK
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Company Name -

Client :GÜVEN SOGUTMA

Vessel Tag No.:OS.C.130B.35.1

Visual Vessel Design by Hexagon PPM,Ver:20.0 Operator : Rev.:A

EN13445:2014 Issue 5:2018+A5 - EN13445; 16.12 - VERTICAL VESSELS WITH SKIRT

SK.1 Skirt 09 Feb. 2021 14:35 ConnID:S1.1

16.12.6.2 Bending Stresses

a) Structural Shape A - figure 16.12-1

Total Bending Moments at Verification Points p and q:

$$M_p = 0.5 * (D_z - DB) * F_{zp} \text{ (16.12-18)} = 0.5 * (114 - 162.41) * -347.46 = 8410.36 \text{ Nmm}$$

$$M_q = 0.5 * (D_z - DB) * F_{zq} \text{ (16.12-19)} = 0.5 * (114 - 162.41) * -519.49 = 12574.16 \text{ Nmm}$$

$$e_{1pm} = ((F_{zp} + dF_g + FF) / (\pi * DB) + p * DB / 4) / f_B \text{ (16.12-7)}$$

$$= ((-347.46 + 0.2571 + 66.31) / (3.14 * 162.41) + 11.7 * 162.41 / 4) / 202.67 = 2.3412 \text{ mm}$$

$$e_{1qm} = ((F_{zq} + dF_g + FF) / (\pi * DB) + p * DB / 4) / f_B \text{ (16.12-8)}$$

$$= ((-519.49 + 0.2571 + 66.31) / (3.14 * 162.41) + 11.7 * 162.41 / 4) / 202.67 = 2.3396 \text{ mm}$$

$$e_{2m} = ((dF_g + FF) / (\pi * DB) + p * DB / 4) / f_B \text{ (16.12-11)}$$

$$= ((0.2571 + 66.31) / (3.14 * 162.41) + 11.7 * 162.41 / 4) / 202.67 = 2.3446 \text{ mm}$$

$$e_{3pm} = (F_{zp} / (\pi * D_z)) / f_B \text{ (16.12-11)}$$

$$= (-347.46 / (3.14 * 114)) / 202.67 = -0.0048 \text{ mm}$$

$$e_{3qm} = (F_{zq} / (\pi * D_z)) / f_B \text{ (16.12-11)}$$

$$= (-519.49 / (3.14 * 114)) / 202.67 = -0.0072 \text{ mm}$$

Total Section Modulus of the Support Ring at Verification Point

$$W_p = \pi / 4 * ((D_z + e_z - DB - e_B) * h^2 + (2 * e_B^2 - e_{1pm}^2 - e_{2m}^2) * DB + 0.5 * (e_{2m}^2 - e_{3pm}^2) * D_z) \text{ (16.12-20)}$$

$$= 3.14 / 4 * ((114 + 3 - 162.41 - 5.39) * 0^2 + (2 * 5.39^2 - 2.34^2 - 2.34^2) * 162.41 + 0.5 * (2.34^2 - (-0.0048)^2) * 114) = 6257.28 \text{ mm}^3$$

$$W_q = \pi / 4 * ((D_z + e_z - DB - e_B) * h^2 + (2 * e_B^2 - e_{1qm}^2 - e_{2m}^2) * DB + 0.5 * (e_{2m}^2 - e_{3qm}^2) * D_z) \text{ (16.12-21)}$$

$$= 3.14 / 4 * ((114 + 3 - 162.41 - 5.39) * 0^2 + (2 * 5.39^2 - 2.34^2 - 2.34^2) * 162.41 + 0.5 * (2.34^2 - (-0.0072)^2) * 114) = 6258.27 \text{ mm}^3$$

16.12.6.3 Total Stresses and Strength Conditions

a) Structural Shape A

1) Location p ABS(M_p)/ $W_p=1.34 \leq f_T=78.33$ [N/mm ²] (16.12-44)	1.7%	OK
2) Location q ABS(M_q)/ $W_q=2.01 \leq f_T=78.33$ [N/mm ²] (16.12-45)	2.5%	OK

16.12.7 Verification of Skirt (Section 4-4)

Note: Skirt is pierced and has a minimum section modulus at $z = 0$ mm

Verification of Skirt at Bottom(no openings) at Elevation $z = -60$ mm

Additional Moment due to Displacement of Neutral Axis

$$dM_4 = \epsilon_s * F_4 = 0 * 433.48 = 0.00 \text{ kNmm}$$

Membrane Stress(Sect.4-4/ $z=-60$)

$$\sigma_{4pm} = (M_{44} + dM_4) / W_4 - F_4 / A_4 = (2688.88 + 0) / 29856.61 - 433.48 / 1074.43 = -0.3134 \text{ N/mm}^2$$

$$\sigma_{4qm} = -1 * (M_{44} + dM_4) / W_4 - F_4 / A_4 = -1 * (2688.88 + 0) / 29856.61 - 433.48 / 1074.43 = -0.4935 \text{ N/mm}^2$$

Membrane Stress(Sect.4-4/ $z=-60$) $\sigma_{4pm}=0.3134 \leq f_z=78.33$ [N/mm ²]	0.4%	OK
Membrane Stress(Sect.4-4/ $z=-60$) $\sigma_{4qm}=0.4935 \leq f_z=78.33$ [N/mm ²]	0.6%	OK

Compressive Stresses in Skirt(Sect.4-4/ $z=-60$)

$$\sigma_{4c} = -F_4 / A_4 = -433.48 / 1074.43 = -0.4034 \text{ N/mm}^2$$

16.14.8 COMPRESSIVE STRESS LIMITS

$$K = 1.21 * E * e_a / (\sigma_{ige} * D) \text{ (16.14-15)}$$

$$= 1.21 * 206070 * 3 / (235 * 114) = 27.92$$

$$\alpha = 0.83 / \sqrt{1 + 0.005 * D / e_a} \text{ (16.14-16)}$$

$$= 0.83 / \sqrt{1 + 0.005 * 114 / 3} = 0.7609$$

$$\Delta = (1 - 0.4123 / (\alpha * K)^{0.6}) / S \text{ (16.14-19)}$$

$$= (1 - 0.4123 / (0.7609 * 27.92)^{0.6}) / 1.5 = 0.6227$$

Maximum Allowable Compressive Stress

$$\sigma_{call} = \sigma_{ige} * \Delta \text{ (16.14-20)} = 235 * 0.6227 = 146.34 \text{ N/mm}^2$$

Company Name -

Client :GÜVEN SOGUTMA

Vessel Tag No.:OS.C.130B.35.1

Visual Vessel Design by Hexagon PPM,Ver:20.0 Operator : Rev.:A

EN13445:2014 Issue 5:2018+A5 - EN13445; 16.12 - VERTICAL VESSELS WITH SKIRT

SK.1 Skirt 09 Feb. 2021 14:35 ConnID:S1.1

Compr.Stress Limits (4-4/z=-60) Sig4c=0.4034 <=
Sigcall=146.34[N/mm2]

0.2%

OK

Check of Skirt Stability/Weakening due to Openings in Skirt

Maximum Compressive Force at Section 4-4

$$Fc4max = \pi * D4 * ez * Sigcall \quad (16.14-2) = 3.14 * 114 * 3 * 146.34 = 157.23 \text{ kN}$$

Maximum Bending Moment at Section 4-4

$$M4max = \pi / 4 * D4^2 * ez * Sigcall \quad (16.14-3) \\ = 3.14 / 4 * 114^2 * 3 * 146.34 = 4.4812 \text{ kNm}$$

Stability of skirt(Sect.4-4/z=-60)

$$\text{StabilityFactor} = F4 / (\Psi1 * Fc4max) + (ABS(M44) + F4 * \text{eps}) / (\Psi2 * M4max) \quad (16.12-70) \\ = 433.48 / (1 * 1.5723E05) + (ABS(2688.88) + 433.48 * 0) / (1 * 4.4812E06) = 0.0034$$

Stability/Openings in skirt(Sect.4-4/z=-60)
StabilityFactor=0.0034 <= 1=1

0.3%

OK

Verification of Skirt at Top(no openings) at Elevation z = -20 mm

Additional Moment due to Displacement of Neutral Axis

$$dM4 = \text{eps} * F4 = 0 * 433.48 = 0.00 \text{ kNm}$$

Membrane Stress(Sect.4-4/z=-20)

$$\text{Sig4pm} = (M44 + dM4) / W4 - F4 / A4 \\ = (2451.32 + 0) / 29856.61 - 433.48 / 1074.43 = -0.3213 \text{ N/mm}^2$$

Sig4qm = - 1 * (M44 + dM4) / W4 - F4 / A4

$$= -1 * (2451.32 + 0) / 29856.61 - 433.48 / 1074.43 = -0.4856 \text{ N/mm}^2$$

Membrane Stress(Sect.4-4/z=-20) Sig4pm=0.3213 <=
fz=78.33[N/mm2]

0.4%

OK

Membrane Stress(Sect.4-4/z=-20) Sig4qm=0.4856 <=
fz=78.33[N/mm2]

0.6%

OK

Compressive Stresses in Skirt(Sect.4-4/z=-20)

$$\text{Sig4c} = - F4 / A4 = -433.48 / 1074.43 = -0.4034 \text{ N/mm}^2$$

16.14.8 COMPRESSIVE STRESS LIMITS

$$K = 1.21 * E * ea / (\text{Sige} * D) \quad (16.14-15) \\ = 1.21 * 206070 * 3 / (235 * 114) = 27.92$$

$$\text{alfa} = 0.83 / \text{Sqr}(1 + 0.005 * D / ea) \quad (16.14-16) \\ = 0.83 / \text{Sqr}(1 + 0.005 * 114 / 3) = 0.7609$$

$$\text{delta} = (1 - 0.4123 / (\text{alfa} * K)^{0.6}) / S \quad (16.14-19) \\ = (1 - 0.4123 / (0.7609 * 27.92)^{0.6}) / 1.5 = 0.6227$$

$$\text{Maximum Allowable Compressive Stress} \\ \text{Sigcall} = \text{Sige} * \text{delta} \quad (16.14-20) = 235 * 0.6227 = 146.34 \text{ N/mm}^2$$

Compr.Stress Limits (4-4/z=-20) Sig4c=0.4034 <=
Sigcall=146.34[N/mm2]

0.2%

OK

Check of Skirt Stability/Weakening due to Openings in Skirt

Maximum Compressive Force at Section 4-4

$$Fc4max = \pi * D4 * ez * Sigcall \quad (16.14-2) = 3.14 * 114 * 3 * 146.34 = 157.23 \text{ kN}$$

Maximum Bending Moment at Section 4-4

$$M4max = \pi / 4 * D4^2 * ez * Sigcall \quad (16.14-3) \\ = 3.14 / 4 * 114^2 * 3 * 146.34 = 4.4812 \text{ kNm}$$

Stability of skirt(Sect.4-4/z=-20)

$$\text{StabilityFactor} = F4 / (\Psi1 * Fc4max) + (ABS(M44) + F4 * \text{eps}) / (\Psi2 * M4max) \quad (16.12-70) \\ = 433.48 / (1 * 1.5723E05) + (ABS(2451.32) + 433.48 * 0) / (1 * 4.4812E06) = 0.0033$$

Stability/Openings in skirt(Sect.4-4/z=-20)
StabilityFactor=0.0033 <= 1=1

0.3%

OK

Company Name -

Client :GÜVEN SOGUTMA

Vessel Tag No.:OS.C.130B.35.1

Visual Vessel Design by Hexagon PPM,Ver:20.0 Operator : Rev.:A

EN13445:2014 Issue 5:2018+A5 - EN13445; 16.12 - VERTICAL VESSELS WITH SKIRT

SK.1 Skirt 09 Feb. 2021 14:35 ConnID:S1.1

SKIRT BASE DESIGN

Wsk = $F4 / (PI * Dz_b) = 433.48 / (3.14 * 114) = 1.2103$ N/mm
Msk = $4 * M4 / (PI * Dz_b^2) = 4 * 2.69 / (3.14 * 114^2) = 0.2634$ N/mm
Outer radial width of bearing plate/base plate
 $b2 = (dob - (Dz_b + ez)) / 2 = (190 - (114 + 3)) / 2 = 36.50$ mm

16.12.5 Anchor Bolt Design

Bolt Circle Diameter
 $d = Dz_b + ea3 + 2 * b3 = 114 + 3 + 2 * 23 = 163.00$ mm
Mean Diameter of Bearing Ring/Base Plate
DCR = $dob - b1 = 190 - 70 = 120.00$ mm
Inside Diameter of Bearing Ring/Base Plate
D3 = $dob - 2 * b1 = 190 - 2 * 70 = 50.00$ mm
Pre-tensioning of bolts FA
FA = $Phi * Ae * fdAnchor(16.12.80) = 0.5 * 36.6 * 181.82 = 3327.27$ N
Maximum anchor bolt force FB
FB = $(4 * M5 / d - F5) / n(16.12.77) = (4 * 2688.88 / 163 - 433.48) / 4 = -91.87$ N
Required minimum torque(per bolt).
Mt = $mu * FA * 1.1 * SQR(4 * Ae / PI) (16.12-81) = 0.2 * 3327.27 * 1.1 * SQR(4 * 36.6 / 3.14) = 4.9970$ Nm
Design anchor bolt load FBd
FBd = $MAX(FA, FB)(16.12.83) = MAX(3327.27, -91.87) = 3327.27$ N
Required Bolting Area
Aemin = $MAX(FB, 0) / fdAnchor = MAX(-91.87, 0) / 181.82 = 0.00$ mm²

Anchor Bolt Area Aemin=0 <= Ae=36.6[mm²] **0.0%** **OK**

Spacing between anchor bolts
 $b8 = PI * (Dz_b + ea3 + 2 * b3) / n = 3.14 * (114 + 3 + 2 * 23) / 4 = 128.02$ mm
Design load on concrete FCd
FCd = $MAX(4 * M5 / DCR + F5, FA * n) = MAX(4 * 2688.88 / 120 + 433.48, 3327.27 * 4) = 13309.09$ N
Foundation Bearing Pressure on concrete below base ring
Sigc = $FCd / (PI * DCR * b1)(16.12-86) = 13309.09 / (3.14 * 120 * 70) = 0.5043$ N/mm²

Foundation Bearing Pressure(concrete) Sigc=0.5043 <= fcd/1.35=5.48[N/mm²] (16.12-86) **9.2%** **OK**

16.12.5.4.3 Checks for type 1 - Simple Bearing Plate

Required minimum analysis thickness of bearing plate (FB <= 0)
 $ea4min = b2 * SQR(3 * Sigc / f4) (16.12-90b) = 36.5 * SQR(3 * 0.5043 / 142.67) = 3.7588$ mm

Min.Thk.of Base Ring ea4min=3.76 <= eR=5[mm] **75.1%** **OK**

LOAD CASE NO: 5 - LC6&7&8 OPER.SEISMIC

Summation of Total Loads for Load Case : LC6&7&8 OPER.SEISMIC

Fz (Force in Vertical Direction)= == -0.5017 kN
Fx (Force in X-Direction)= == 0.0975 kN
Fy (Force in Y-Direction)= == 0.00 kN
Fs= $SQR(Fx^2 + Fy^2)$ (Tot.Force in Hor.Plane)= == 0.0975 kN
Moments at top of skirt at elevation of shell/skirt connection.
Mx (Moment around the X-Axis)= == -1.28 kNm
My (Moment around the Y-Axis)= == 50.21 kNm
M1= $SQR(Mx^2 + My^2)$ (Moment in XY Plane)= == 50.22 kNm
Mt (Moment around the z-axis)= == 0.5749 kNm
Moments at elevation at bottom of skirt.
Mx4 (Moment around the X-Axis)= == -1.28 kNm
My4 (Moment around the Y-Axis)= == 54.21 kNm
M4= $SQR(Mx4^2 + My4^2)$ (Moment in XY Plane)= == 54.22 kNm
FF (Weight of Content of Vessel)= == 0.0663 kN

Company Name -

Client :GÜVEN SOGUTMA

Vessel Tag No.:OS.C.130B.35.1

Visual Vessel Design by Hexagon PPM,Ver:20.0 Operator : Rev.:A

EN13445:2014 Issue 5:2018+A5 - EN13445; 16.12 - VERTICAL VESSELS WITH SKIRT

SK.1 Skirt 09 Feb. 2021 14:35 ConnID:S1.1

dFg (Weight of Vessel Below Section 2-2)= (16.12-3) == 2,6851E-04 kN

16.12.6.1 Verification of Membrane Stresses

Fzp = Fz + 4 * M1 / Dz (16.12-1) = -501.7+4*50.22/114= 1260.55 N

Fzq = Fz - 4 * M1 / Dz (16.12-2) = -501.7-4*50.22/114= -2264.01 N

16.14.8 COMPRESSIVE STRESS LIMITS

K = 1.21 * E * ea / (Sige * D) (16.14-15)

=1.21*2.0607E05*5.39/(304*162.41)= 27.22

alfa = 0.83 / Sqr(1 + 0.005 * D / ea) (16.14-16)

=0.83/Sqr(1+0.005*162.41/5.39)= 0.7738

delta = (1 - 0.4123 / (alfa * K) ^ 0.6) / S (16.14-19)

=(1-0.4123/(0.7738*27.22)^0.6)/1.5= 0.6225

Maximum Allowable Compressive Stress

Sigcall = Sige * delta (16.14-20) =304*0.6225= 189.24 N/mm2

Stresses in section 1-1 (internal pressure)

Membrane Stress(Sect.1-1)

Sig1pm = (Fzp+dFg+FF)/(PI*DB*eB)+p*DB/(4*eB)
 =(1260.55+0.2685+66.31)/(3.14*162.41*5.39)+11.7*162.41/(4*5.39)= 88.62 N/mm2

Sig1qm = (Fzq+dFg+FF)/(PI*DB*eB)+p*DB/(4*eB)
 =(-2264.01+0.2685+66.31)/(3.14*162.41*5.39)+11.7*162.41/(4*5.39)
 = 87.34 N/mm2

Membrane Stress(Pint)(Sect.1-1) Sig1pm=88.62 <= fB=288.8[N/mm2]	30.6%	OK
---	-------	----

Membrane Stress(Pint)(Sect.1-1) ABS(Sig1qm)=87.34 <= fB=288.8[N/mm2]	30.2%	OK
--	-------	----

Stresses in section 1-1 (external pressure)

Membrane Stress(Sect.1-1)

Sig1pmext = (Fzp+dFg+FF)/(PI*DB*eB)-Pext*DB/(4*eB)
 =(1260.55+0.2685+66.31)/(3.14*162.41*5.39)-0*162.41/(4*5.39)= 0.4826 N/mm2

Sig1qmext = (Fzq+dFg+FF)/(PI*DB*eB)-Pext*DB/(4*eB)
 =(-2264.01+0.2685+66.31)/(3.14*162.41*5.39)-0*162.41/(4*5.39)= -0.7990 N/mm2

Membrane Stress(Pext)(Sect.1-1) Sig1pmext=0.4826 <= fB=288.8[N/mm2]	0.1%	OK
---	------	----

Compr.Stress(Pext)(Sect.1-1) Sig1qmext=0.799 <= Sigcall=227.09[N/mm2]	0.3%	OK
---	------	----

Membrane Stress(Sect.2-2)

Sig2pm = Sig2qm=(FF+dFg)/(PI*DB*eB)+p*DB/(4*eB)
 =(66.31+0.2685)/(3.14*162.41*5.39)+11.7*162.41/(4*5.39)= 88.16 N/mm2

Membrane Stress(Sect.2-2) ABS(Sig2pm)=88.16 <= fB=288.8[N/mm2]	30.5%	OK
--	-------	----

Membrane Stress(Sect.3-3)

Sig3pm = Fzp / (PI * Dz * ez) =1260.55/(3.14*114*3)= 1.1732 N/mm2

Sig3qm = Fzq / (PI * Dz * ez) =-2264.01/(3.14*114*3)= -2.11 N/mm2

Membrane Stress(Sect.3-3) ABS(Sig3pm)=1.17 <= fz=111.62[N/mm2]	1.0%	OK
--	------	----

Membrane Stress(Sect.3-3) ABS(Sig3qm)=2.11 <= fz=111.62[N/mm2]	1.8%	OK
--	------	----

Company Name -

Client :GÜVEN SOGUTMA

Vessel Tag No.:OS.C.130B.35.1

Visual Vessel Design by Hexagon PPM,Ver:20.0 Operator : Rev.:A

EN13445:2014 Issue 5:2018+A5 - EN13445; 16.12 - VERTICAL VESSELS WITH SKIRT

SK.1 Skirt 09 Feb. 2021 14:35 ConnID:S1.1

16.12.6.2 Bending Stresses

a) Structural Shape A - figure 16.12-1

Total Bending Moments at Verification Points p and q:

$$M_p = 0.5 * (D_z - DB) * F_{zp} \text{ (16.12-18)} = 0.5 * (114 - 162.41) * 1260.55 = -30511.62 \text{ Nmm}$$

$$M_q = 0.5 * (D_z - DB) * F_{zq} \text{ (16.12-19)} = 0.5 * (114 - 162.41) * (-2264.01) = 54800.31 \text{ Nmm}$$

$$e_{1pm} = ((F_{zp} + dF_g + FF) / (\pi * DB) + p * DB / 4) / f_B \text{ (16.12-7)}$$

$$= ((1260.55 + 0.2685 + 66.31) / (3.14 * 162.41) + 11.7 * 162.41 / 4) / 288.8 = 1.6539 \text{ mm}$$

$$e_{1qm} = ((F_{zq} + dF_g + FF) / (\pi * DB) + p * DB / 4) / f_B \text{ (16.12-8)}$$

$$= ((-2264.01 + 0.2685 + 66.31) / (3.14 * 162.41) + 11.7 * 162.41 / 4) / 288.8 = 1.6300 \text{ mm}$$

$$e_{2m} = ((dF_g + FF) / (\pi * DB) + p * DB / 4) / f_B \text{ (16.12-11)}$$

$$= ((0.2685 + 66.31) / (3.14 * 162.41) + 11.7 * 162.41 / 4) / 288.8 = 1.6453 \text{ mm}$$

$$e_{3pm} = (F_{zp} / (\pi * D_z)) / f_B \text{ (16.12-11)}$$

$$= (1260.55 / (3.14 * 114)) / 288.8 = 0.0122 \text{ mm}$$

$$e_{3qm} = (F_{zq} / (\pi * D_z)) / f_B \text{ (16.12-11)}$$

$$= (-2264.01 / (3.14 * 114)) / 288.8 = -0.0219 \text{ mm}$$

Total Section Modulus of the Support Ring at Verification Point

$$W_p = \pi / 4 * ((D_z + e_z - DB - e_B) * h^2 + (2 * e_B^2 - e_{1pm}^2 - e_{2m}^2) * DB + 0.5 * (e_{2m}^2 - e_{3pm}^2) * D_z) \text{ (16.12-20)}$$

$$= 3.14 / 4 * ((114 + 3 - 162.41 - 5.39) * 0^2 + (2 * 5.39^2 - 1.65^2 - 1.65^2) * 162.41 + 0.5 * (1.65^2 - 0.0122^2) * 114) = 6838.53 \text{ mm}^3$$

$$W_q = \pi / 4 * ((D_z + e_z - DB - e_B) * h^2 + (2 * e_B^2 - e_{1qm}^2 - e_{2m}^2) * DB + 0.5 * (e_{2m}^2 - e_{3qm}^2) * D_z) \text{ (16.12-21)}$$

$$= 3.14 / 4 * ((114 + 3 - 162.41 - 5.39) * 0^2 + (2 * 5.39^2 - 1.63^2 - 1.65^2) * 162.41 + 0.5 * (1.65^2 - 0.0219^2) * 114) = 6848.54 \text{ mm}^3$$

16.12.6.3 Total Stresses and Strength Conditions

a) Structural Shape A

1) Location p ABS(M_p)/ $W_p=4.46 \leq f_T=111.62$ [N/mm ²] (16.12-44)	3.9%	OK
2) Location q ABS(M_q)/ $W_q=8. \leq f_T=111.62$ [N/mm ²] (16.12-45)	7.1%	OK

16.12.7 Verification of Skirt (Section 4-4)

Note: Skirt is pierced and has a minimum section modulus at $z = 0$ mm

Verification of Skirt at Bottom(no openings) at Elevation $z = -60$ mm

Additional Moment due to Displacement of Neutral Axis

$$dM_4 = \epsilon_p * F_4 = 0 * 501.73 = 0.00 \text{ kNm}$$

Membrane Stress(Sect.4-4/ $z=-60$)

$$\sigma_{4pm} = (M_{44} + dM_4) / W_4 - F_4 / A_4 = (54220.32 + 0) / 29856.61 - 501.73 / 1074.43 = 1.3490 \text{ N/mm}^2$$

$\sigma_{4qm} = -1 * (M_{44} + dM_4) / W_4 - F_4 / A_4$

$$= -1 * (54220.32 + 0) / 29856.61 - 501.73 / 1074.43 = -2.28 \text{ N/mm}^2$$

Membrane Stress(Sect.4-4/ $z=-60$) $\sigma_{4pm}=1.35 \leq f_z=111.62$ [N/mm ²]	1.2%	OK
Membrane Stress(Sect.4-4/ $z=-60$) $\sigma_{4qm}=2.28 \leq f_z=111.62$ [N/mm ²]	2.0%	OK

Compressive Stresses in Skirt(Sect.4-4/ $z=-60$)

$$\sigma_{4c} = -F_4 / A_4 = -501.73 / 1074.43 = -0.4670 \text{ N/mm}^2$$

16.14.8 COMPRESSIVE STRESS LIMITS

$$K = 1.21 * E * e_a / (\sigma_{ige} * D) \text{ (16.14-15)}$$

$$= 1.21 * 206070 * 3 / (235 * 114) = 27.92$$

$$\alpha = 0.83 / \sqrt{1 + 0.005 * D / e_a} \text{ (16.14-16)}$$

$$= 0.83 / \sqrt{1 + 0.005 * 114 / 3} = 0.7609$$

$$\delta = (1 - 0.4123 / (\alpha * K))^{0.6} / S \text{ (16.14-19)}$$

$$= (1 - 0.4123 / (0.7609 * 27.92))^{0.6} / 1.5 = 0.6227$$

Maximum Allowable Compressive Stress

$$\sigma_{call} = \sigma_{ige} * \delta \text{ (16.14-20)} = 235 * 0.6227 = 146.34 \text{ N/mm}^2$$

Company Name -

Client :GÜVEN SOGUTMA

Vessel Tag No.:OS.C.130B.35.1

Visual Vessel Design by Hexagon PPM,Ver:20.0 Operator : Rev.:A

EN13445:2014 Issue 5:2018+A5 - EN13445; 16.12 - VERTICAL VESSELS WITH SKIRT

SK.1 Skirt 09 Feb. 2021 14:35 ConnID:S1.1

Compr.Stress Limits (4-4/z=-60) Sig4c=0.467 <= Sigcall=175.61[N/mm2]	0.2%	OK
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Check of Skirt Stability/Weakening due to Openings in Skirt

Maximum Compressive Force at Section 4-4

$$F_{c4max} = \pi * D_4 * e_z * \text{Sigcall} \quad (16.14-2) = 3.14 * 114 * 3 * 146.34 = 188.68 \text{ kN}$$

Maximum Bending Moment at Section 4-4

$$M_{4max} = \pi / 4 * D_4^2 * e_z * \text{Sigcall} \quad (16.14-3) \\ = 3.14 / 4 * 114^2 * 3 * 146.34 = 5.3774 \text{ kNm}$$

Stability of skirt(Sect.4-4/z=-60)

$$\text{StabilityFactor} = F_4 / (\text{Psi1} * F_{c4max}) + (\text{ABS}(M_{44}) + F_4 * \text{eps}) / (\text{Psi2} * M_{4max}) \quad (16.12-70) \\ = 501.73 / (1 * 1.8868E05) + (\text{ABS}(54220.32) + 501.73 * 0) / (1 * 5.3774E06) = 0.0127$$

Stability/Openings in skirt(Sect.4-4/z=-60) StabilityFactor=0.0127 <= 1=1	1.2%	OK
---	-------------	-----------

Verification of Skirt at Top(no openings) at Elevation z = -20 mm

Additional Moment due to Displacement of Neutral Axis

$$dM_4 = \text{eps} * F_4 = 0 * 501.73 = 0.00 \text{ kNm}$$

Membrane Stress(Sect.4-4/z=-20)

$$\text{Sig4pm} = (M_{44} + dM_4) / W_4 - F_4 / A_4 \\ = (50224.95 + 0) / 29856.61 - 501.73 / 1074.43 = 1.2152 \text{ N/mm}^2$$

Sig4qm = - 1 * (M44 + dM4) / W4 - F4 / A4

$$= -1 * (50224.95 + 0) / 29856.61 - 501.73 / 1074.43 = -2.15 \text{ N/mm}^2$$

Membrane Stress(Sect.4-4/z=-20) Sig4pm=1.22 <= fz=111.62[N/mm2]	1.0%	OK
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Membrane Stress(Sect.4-4/z=-20) Sig4qm=2.15 <= fz=111.62[N/mm2]	1.9%	OK
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Compressive Stresses in Skirt(Sect.4-4/z=-20)

$$\text{Sig4c} = - F_4 / A_4 = -501.73 / 1074.43 = -0.4670 \text{ N/mm}^2$$

16.14.8 COMPRESSIVE STRESS LIMITS

$$K = 1.21 * E * e_a / (\text{Sige} * D) \quad (16.14-15) \\ = 1.21 * 206070 * 3 / (235 * 114) = 27.92$$

$$\text{alfa} = 0.83 / \text{Sqr}(1 + 0.005 * D / e_a) \quad (16.14-16) \\ = 0.83 / \text{Sqr}(1 + 0.005 * 114 / 3) = 0.7609$$

$$\text{delta} = (1 - 0.4123 / (\text{alfa} * K)^{0.6}) / S \quad (16.14-19) \\ = (1 - 0.4123 / (0.7609 * 27.92)^{0.6}) / 1.5 = 0.6227$$

$$\text{Maximum Allowable Compressive Stress} \\ \text{Sigcall} = \text{Sige} * \text{delta} \quad (16.14-20) = 235 * 0.6227 = 146.34 \text{ N/mm}^2$$

Compr.Stress Limits (4-4/z=-20) Sig4c=0.467 <= Sigcall=175.61[N/mm2]	0.2%	OK
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Check of Skirt Stability/Weakening due to Openings in Skirt

Maximum Compressive Force at Section 4-4

$$F_{c4max} = \pi * D_4 * e_z * \text{Sigcall} \quad (16.14-2) = 3.14 * 114 * 3 * 146.34 = 188.68 \text{ kN}$$

Maximum Bending Moment at Section 4-4

$$M_{4max} = \pi / 4 * D_4^2 * e_z * \text{Sigcall} \quad (16.14-3) \\ = 3.14 / 4 * 114^2 * 3 * 146.34 = 5.3774 \text{ kNm}$$

Stability of skirt(Sect.4-4/z=-20)

$$\text{StabilityFactor} = F_4 / (\text{Psi1} * F_{c4max}) + (\text{ABS}(M_{44}) + F_4 * \text{eps}) / (\text{Psi2} * M_{4max}) \quad (16.12-70) \\ = 501.73 / (1 * 1.8868E05) + (\text{ABS}(50224.95) + 501.73 * 0) / (1 * 5.3774E06) = 0.0120$$

Stability/Openings in skirt(Sect.4-4/z=-20) StabilityFactor=0.012 <= 1=1	1.1%	OK
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Company Name -

Client :GÜVEN SOGUTMA

Vessel Tag No.:OS.C.130B.35.1

Visual Vessel Design by Hexagon PPM,Ver:20.0 Operator : Rev.:A

EN13445:2014 Issue 5:2018+A5 - EN13445; 16.12 - VERTICAL VESSELS WITH SKIRT

SK.1 Skirt 09 Feb. 2021 14:35 ConnID:S1.1

SKIRT BASE DESIGN

Wsk = $F4 / (PI * Dz_b) = 501.73 / (3.14 * 114) = 1.4009$ N/mm
Msk = $4 * M4 / (PI * Dz_b^2) = 4 * 54.22 / (3.14 * 114^2) = 5.3121$ N/mm
Outer radial width of bearing plate/base plate
 $b2 = (dob - (Dz_b + ez)) / 2 = (190 - (114 + 3)) / 2 = 36.50$ mm

16.12.5 Anchor Bolt Design

Bolt Circle Diameter
 $d = Dz_b + ea3 + 2 * b3 = 114 + 3 + 2 * 23 = 163.00$ mm
Mean Diameter of Bearing Ring/Base Plate
DCR = $dob - b1 = 190 - 70 = 120.00$ mm
Inside Diameter of Bearing Ring/Base Plate
D3 = $dob - 2 * b1 = 190 - 2 * 70 = 50.00$ mm
Pre-tensioning of bolts FA
FA = $Phi * Ae * fdAnchor (16.12.80) = 0.5 * 36.6 * 218.18 = 3992.73$ N
Maximum anchor bolt force FB
FB = $(4 * M5 / d - F5) / n (16.12.77) = (4 * 54220.32 / 163 - 501.73) / 4 = 207.21$ N
Required minimum torque(per bolt).
Mt = $mu * FA * 1.1 * SQR(4 * Ae / PI) (16.12-81) = 0.2 * 3992.73 * 1.1 * SQR(4 * 36.6 / 3.14) = 5.9964$ Nm
Design anchor bolt load FBd
FBd = $MAX(FA, FB) (16.12.83) = MAX(3992.73, 207.21) = 3992.73$ N
Required Bolting Area
Aemin = $MAX(FB, 0) / fdAnchor = MAX(207.21, 0) / 218.18 = 0.9497$ mm²

Anchor Bolt Area Aemin=0.9497 <= Ae=36.6[mm²] 2.5% OK

Spacing between anchor bolts
 $b8 = PI * (Dz_b + ea3 + 2 * b3) / n = 3.14 * (114 + 3 + 2 * 23) / 4 = 128.02$ mm
Design load on concrete FCd
FCd = $MAX(4 * M5 / DCR + F5, FA * n) = MAX(4 * 54220.32 / 120 + 501.73, 3992.73 * 4) = 15970.91$ N
Foundation Bearing Pressure on concrete below base ring
Sigc = $FCd / (PI * DCR * b1) (16.12-86) = 15970.91 / (3.14 * 120 * 70) = 0.6052$ N/mm²

Foundation Bearing Pressure(concrete) Sigc=0.6052 <= fcd/1.35=5.48[N/mm²] (16.12-86) 11.0% OK

16.12.5.4.3 Checks for type 1 - Simple Bearing Plate

Required minimum analysis thickness of bearing plate (FB > 0)
 $ea4min = MAX(b2 * SQR(3 * Sigc / f4), SQR(4 * n * FB * b3 / (PI * D3 * f4))) (16.12-90a) = MAX(36.5 * SQR(3 * 0.6052 / 203.3), SQR(4 * 4 * 207.21 * 23 / (3.14 * 50 * 203.3))) = 3.4493$ mm

Min.Thk.of Base Ring ea4min=3.45 <= eR=5[mm] 68.9% OK

CALCULATION SUMMARY

VESSEL DEFLECTION LOAD CASE NO: 1 - LC9 HYDROTEST

»LC9 HYDROTEST, Max.deflection over 200mm length dallow=1 >= dactual=6.259E-05[mm] «»
OK«

Between z1= 508 and z2= 705 in component:E5.1
Deflection at top of vessel : 0.0 mm

VESSEL DEFLECTION LOAD CASE NO: 2 - LC4 SHUT DOWN

»LC4 SHUT DOWN, Max.deflection over 200mm length dallow=1 >= dactual=1.0323E-04[mm] «»
OK«

Between z1= 484 and z2= 682 in component:F.2
Deflection at top of vessel : 0.0 mm

Company Name -

Client :GÜVEN SOGUTMA

Vessel Tag No.:OS.C.130B.35.1

Visual Vessel Design by Hexagon PPM,Ver:20.0 Operator : Rev.:A

EN13445:2014 Issue 5:2018+A5 - EN13445; 16.12 - VERTICAL VESSELS WITH SKIRT

SK.1 Skirt 09 Feb. 2021 14:35 ConnID:S1.1

VESSEL DEFLECTION LOAD CASE NO: 3 - LC5 INSTALLATION»LC5 INSTALLATION, Max.deflection over 200mm length dallow=1 >= dactual=7.1617E-05[mm] «»
OK«

Between z1= 508 and z2= 705 in component:E5.1

Deflection at top of vessel : 0.0 mm

VESSEL DEFLECTION LOAD CASE NO: 4 - LC1&2&3 OPER.WIND»LC1&2&3 OPER.WIND, Max.deflection over 200mm length dallow=1 >= dactual=1.0323E-04[mm]
«» OK«

Between z1= 484 and z2= 682 in component:F.2

Deflection at top of vessel : 0.0 mm

VESSEL DEFLECTION LOAD CASE NO: 5 - LC6&7&8 OPER.SEISMIC»LC6&7&8 OPER.SEISMIC, Max.deflection over 200mm length dallow=1 >= dactual=0.0023[mm] «»
OK«

Between z1= 484 and z2= 682 in component:F.2

Deflection at top of vessel : 0.0 mm

LOAD CASE NO: 1 - LC9 HYDROTEST

16.12.6.1 Verification of Membrane Stresses

Membrane Stress(Pint)(Sect.1-1) Sig1pm=128.04 <= fB=338.1[N/mm2]	37.8%	OK
Membrane Stress(Pint)(Sect.1-1) ABS(Sig1qm)=128. <= fB=338.1[N/mm2]	37.8%	OK
Compr.Stress(Pext)(Sect.1-1) Sig1pmext=0.1094 <= Sigcall=315.25[N/mm2]	0.0%	OK
Compr.Stress(Pext)(Sect.1-1) Sig1qmext=0.1463 <= Sigcall=315.25[N/mm2]	0.0%	OK
Membrane Stress(Sect.2-2) ABS(Sig2pm)=128.19 <= fB=338.1[N/mm2]	37.9%	OK
Membrane Stress(Sect.3-3) ABS(Sig3pm)=0.4256 <= fz=117.5[N/mm2]	0.3%	OK
Membrane Stress(Sect.3-3) ABS(Sig3qm)=0.5289 <= fz=117.5[N/mm2]	0.4%	OK

16.12.6.2 Bending Stresses

1) Location p ABS(Mp)/Wp=1.42 <= fT=117.5[N/mm2] (16.12-44)	1.2%	OK
2) Location q ABS(Mq)/Wq=1.76 <= fT=117.5[N/mm2] (16.12-45)	1.5%	OK

16.12.7 Verification of Skirt (Section 4-4)

Membrane Stress(Sect.4-4/z=-60) Sig4pm=0.4204 <= fz=117.5[N/mm2]	0.3%	OK
Membrane Stress(Sect.4-4/z=-60) Sig4qm=0.5341 <= fz=117.5[N/mm2]	0.4%	OK
Compr.Stress Limits (4-4/z=-60) Sig4c=0.4773 <= Sigcall=209.06[N/mm2]	0.2%	OK

Company Name -

Client :GÜVEN SOGUTMA

Vessel Tag No.:OS.C.130B.35.1

Visual Vessel Design by Hexagon PPM,Ver:20.0 Operator : Rev.:A

EN13445:2014 Issue 5:2018+A5 - EN13445; 16.12 - VERTICAL VESSELS WITH SKIRT

SK.1 Skirt 09 Feb. 2021 14:35 ConnID:S1.1

Check of Skirt Stability/Weakening due to Openings in Skirt

Stability/Openings in skirt(Sect.4-4/z=-60) StabilityFactor=0.0025 <= 1=1	0.2%	OK
Membrane Stress(Sect.4-4/z=-20) Sig4pm=0.4243 <= fz=117.5[N/mm2]	0.3%	OK
Membrane Stress(Sect.4-4/z=-20) Sig4qm=0.5302 <= fz=117.5[N/mm2]	0.4%	OK
Compr.Stress Limits (4-4/z=-20) Sig4c=0.4773 <= Sigcall=209.06[N/mm2]	0.2%	OK

Check of Skirt Stability/Weakening due to Openings in Skirt

Stability/Openings in skirt(Sect.4-4/z=-20) StabilityFactor=0.0025 <= 1=1	0.2%	OK
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SKIRT BASE DESIGN

Anchor Bolt Area Aemin=0 <= Ae=36.6[mm2]	0.0%	OK
Foundation Bearing Pressure(concrete) Sigc=0.5043 <= fcd/1.35=5.48[N/mm2] (16.12-86)	9.2%	OK
Required minimum analysis thickness of bearing plate (FB <= 0) ea4min = b2 * SQR(3 * SigC / f4) (16.12-90b) =36.5*SQR(3*0.5043/223.81)= 3.0011 mm		
Min.Thk.of Base Ring ea4min=3. <= eR=5[mm]	60.0%	OK

LOAD CASE NO: 2 - LC4 SHUT DOWN

16.12.6.1 Verification of Membrane Stresses

Compr.Stress(Pint)(Sect.1-1) Sig1pm=0.1021 <= Sigcall=189.24[N/mm2]	0.0%	OK
Compr.Stress(Pint)(Sect.1-1) Sig1qm=0.1647 <= Sigcall=189.24[N/mm2]	0.0%	OK
Compr.Stress(Pext)(Sect.1-1) Sig1pmext=0.1021 <= Sigcall=189.24[N/mm2]	0.0%	OK
Compr.Stress(Pext)(Sect.1-1) Sig1qmext=0.1647 <= Sigcall=189.24[N/mm2]	0.0%	OK
Membrane Stress(Sect.2-2) ABS(Sig2pm)=9.3477E-05 <= fB=202.67[N/mm2]	0.0%	OK
Membrane Stress(Sect.3-3) ABS(Sig3pm)=0.2617 <= fz=78.33[N/mm2]	0.3%	OK
Membrane Stress(Sect.3-3) ABS(Sig3qm)=0.4218 <= fz=78.33[N/mm2]	0.5%	OK

16.12.6.2 Bending Stresses

1) Location p ABS(Mp)/Wp=0.9182 <= fT=78.33[N/mm2] (16.12-44)	1.1%	OK
2) Location q ABS(Mq)/Wq=1.48 <= fT=78.33[N/mm2] (16.12- 45)	1.8%	OK

Company Name -

Client :GÜVEN SOGUTMA

Vessel Tag No.:OS.C.130B.35.1

Visual Vessel Design by Hexagon PPM,Ver:20.0 Operator : Rev.:A

EN13445:2014 Issue 5:2018+A5 - EN13445; 16.12 - VERTICAL VESSELS WITH SKIRT

SK.1 Skirt 09 Feb. 2021 14:35 ConnID:S1.1

16.12.7 Verification of Skirt (Section 4-4)

Membrane Stress(Sect.4-4/z=-60) Sig4pm=0.2517 <= fz=78.33[N/mm2]	0.3%	OK
Membrane Stress(Sect.4-4/z=-60) Sig4qm=0.4318 <= fz=78.33[N/mm2]	0.5%	OK
Compr.Stress Limits (4-4/z=-60) Sig4c=0.3417 <= Sigcall=146.34[N/mm2]	0.2%	OK

Check of Skirt Stability/Weakening due to Openings in Skirt

Stability/Openings in skirt(Sect.4-4/z=-60) StabilityFactor=0.0029 <= 1=1	0.2%	OK
Membrane Stress(Sect.4-4/z=-20) Sig4pm=0.2596 <= fz=78.33[N/mm2]	0.3%	OK
Membrane Stress(Sect.4-4/z=-20) Sig4qm=0.4238 <= fz=78.33[N/mm2]	0.5%	OK
Compr.Stress Limits (4-4/z=-20) Sig4c=0.3417 <= Sigcall=146.34[N/mm2]	0.2%	OK

Check of Skirt Stability/Weakening due to Openings in Skirt

Stability/Openings in skirt(Sect.4-4/z=-20) StabilityFactor=0.0029 <= 1=1	0.2%	OK
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SKIRT BASE DESIGN

Anchor Bolt Area Amin=0 <= Ae=36.6[mm2]	0.0%	OK
Foundation Bearing Pressure(concrete) Sigc=0.5043 <= fcd/1.35=5.48[N/mm2] (16.12-86)	9.2%	OK
Required minimum analysis thickness of bearing plate (FB <= 0) ea4min = b2 * SQR(3 * SigC / f4) (16.12-90b) =36.5*SQR(3*0.5043/142.67)= 3.7588 mm		
Min.Thk.of Base Ring ea4min=3.76 <= eR=5[mm]	75.1%	OK

LOAD CASE NO: 3 - LC5 INSTALLATION

16.12.6.1 Verification of Membrane Stresses

Compr.Stress(Pint)(Sect.1-1) Sig1pm=0.1111 <= Sigcall=219.74[N/mm2]	0.0%	OK
Compr.Stress(Pint)(Sect.1-1) Sig1qm=0.1557 <= Sigcall=219.74[N/mm2]	0.0%	OK
Compr.Stress(Pext)(Sect.1-1) Sig1pmext=0.1111 <= Sigcall=219.74[N/mm2]	0.0%	OK
Compr.Stress(Pext)(Sect.1-1) Sig1qmext=0.1557 <= Sigcall=219.74[N/mm2]	0.0%	OK
Membrane Stress(Sect.2-2) ABS(Sig2pm)=9.3477E-05 <= fB=204.17[N/mm2]	0.0%	OK
Membrane Stress(Sect.3-3) ABS(Sig3pm)=0.2847 <= fz=78.33[N/mm2]	0.3%	OK

Company Name -

Client :GÜVEN SOGUTMA

Vessel Tag No.:OS.C.130B.35.1

Visual Vessel Design by Hexagon PPM,Ver:20.0 Operator : Rev.:A

EN13445:2014 Issue 5:2018+A5 - EN13445; 16.12 - VERTICAL VESSELS WITH SKIRT

SK.1 Skirt 09 Feb. 2021 14:35 ConnID:S1.1

Membrane Stress(Sect.3-3) ABS(Sig3qm)=0.3988 <= fz=78.33[N/mm2]	0.5%	OK
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16.12.6.2 Bending Stresses

1) Location p ABS(Mp)/Wp=0.999 <= fT=78.33[N/mm2] (16.12-44)	1.2%	OK
2) Location q ABS(Mq)/Wq=1.4 <= fT=78.33[N/mm2] (16.12-45)	1.7%	OK

16.12.7 Verification of Skirt (Section 4-4)

Membrane Stress(Sect.4-4/z=-60) Sig4pm=0.2785 <= fz=78.33[N/mm2]	0.3%	OK
Membrane Stress(Sect.4-4/z=-60) Sig4qm=0.4049 <= fz=78.33[N/mm2]	0.5%	OK
Compr.Stress Limits (4-4/z=-60) Sig4c=0.3417 <= Sigcall=146.34[N/mm2]	0.2%	OK

Check of Skirt Stability/Weakening due to Openings in Skirt

Stability/Openings in skirt(Sect.4-4/z=-60) StabilityFactor=0.0028 <= 1=1	0.2%	OK
Membrane Stress(Sect.4-4/z=-20) Sig4pm=0.2832 <= fz=78.33[N/mm2]	0.3%	OK
Membrane Stress(Sect.4-4/z=-20) Sig4qm=0.4002 <= fz=78.33[N/mm2]	0.5%	OK
Compr.Stress Limits (4-4/z=-20) Sig4c=0.3417 <= Sigcall=146.34[N/mm2]	0.2%	OK

Check of Skirt Stability/Weakening due to Openings in Skirt

Stability/Openings in skirt(Sect.4-4/z=-20) StabilityFactor=0.0027 <= 1=1	0.2%	OK
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SKIRT BASE DESIGN

Anchor Bolt Area Aemin=0 <= Ae=36.6[mm2]	0.0%	OK
Foundation Bearing Pressure(concrete) Sigc=0.5043 <= fcd/1.35=5.48[N/mm2] (16.12-86)	9.2%	OK

Required minimum analysis thickness of bearing plate (FB <= 0)

$$ea_{4min} = b_2 * SQR(3 * Sigc / f_4)$$

$$= 36.5 * SQR(3 * 0.5043 / 150) =$$

(16.12-90b)

$$3.6658 \text{ mm}$$

Min.Thk.of Base Ring ea4min=3.67 <= eR=5[mm]	73.3%	OK
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LOAD CASE NO: 4 - LC1&2&3 OPER.WIND

16.12.6.1 Verification of Membrane Stresses

Membrane Stress(Pint)(Sect.1-1) Sig1pm=88.03 <= fB=202.67[N/mm2]	43.4%	OK
Membrane Stress(Pint)(Sect.1-1) ABS(Sig1qm)=87.97 <= fB=202.67[N/mm2]	43.4%	OK
Compr.Stress(Pext)(Sect.1-1) Sig1pmext=0.1021 <= Sigcall=189.24[N/mm2]	0.0%	OK

Company Name -

Client :GÜVEN SOGUTMA

Vessel Tag No.:OS.C.130B.35.1

Visual Vessel Design by Hexagon PPM,Ver:20.0 Operator : Rev.:A

EN13445:2014 Issue 5:2018+A5 - EN13445; 16.12 - VERTICAL VESSELS WITH SKIRT

SK.1 Skirt 09 Feb. 2021 14:35 ConnID:S1.1

Compr.Stress(Pext)(Sect.1-1) Sig1qmext=0.1647 <= Sigcall=189.24[N/mm2]	0.0%	OK
Membrane Stress(Sect.2-2) ABS(Sig2pm)=88.16 <= fB=202.67[N/mm2]	43.4%	OK
Membrane Stress(Sect.3-3) ABS(Sig3pm)=0.3234 <= fz=78.33[N/mm2]	0.4%	OK
Membrane Stress(Sect.3-3) ABS(Sig3qm)=0.4835 <= fz=78.33[N/mm2]	0.6%	OK

16.12.6.2 Bending Stresses

1) Location p ABS(Mp)/Wp=1.34 <= fT=78.33[N/mm2] (16.12-44)	1.7%	OK
2) Location q ABS(Mq)/Wq=2.01 <= fT=78.33[N/mm2] (16.12-45)	2.5%	OK

16.12.7 Verification of Skirt (Section 4-4)

Membrane Stress(Sect.4-4/z=-60) Sig4pm=0.3134 <= fz=78.33[N/mm2]	0.4%	OK
Membrane Stress(Sect.4-4/z=-60) Sig4qm=0.4935 <= fz=78.33[N/mm2]	0.6%	OK
Compr.Stress Limits (4-4/z=-60) Sig4c=0.4034 <= Sigcall=146.34[N/mm2]	0.2%	OK

Check of Skirt Stability/Weakening due to Openings in Skirt

Stability/Openings in skirt(Sect.4-4/z=-60) StabilityFactor=0.0034 <= 1=1	0.3%	OK
Membrane Stress(Sect.4-4/z=-20) Sig4pm=0.3213 <= fz=78.33[N/mm2]	0.4%	OK
Membrane Stress(Sect.4-4/z=-20) Sig4qm=0.4856 <= fz=78.33[N/mm2]	0.6%	OK
Compr.Stress Limits (4-4/z=-20) Sig4c=0.4034 <= Sigcall=146.34[N/mm2]	0.2%	OK

Check of Skirt Stability/Weakening due to Openings in Skirt

Stability/Openings in skirt(Sect.4-4/z=-20) StabilityFactor=0.0033 <= 1=1	0.3%	OK
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SKIRT BASE DESIGN

Anchor Bolt Area Aemin=0 <= Ae=36.6[mm2]	0.0%	OK
Foundation Bearing Pressure(concrete) Sigc=0.5043 <= fcd/1.35=5.48[N/mm2] (16.12-86)	9.2%	OK

Required minimum analysis thickness of bearing plate (FB <= 0)

$$ea4min = b2 * \text{SQR}(3 * \text{SigC} / f4)$$

$$= 36.5 * \text{SQR}(3 * 0.5043 / 142.67) =$$

$$(16.12-90b) \\ \underline{\underline{3.7588 \text{ mm}}}$$

Min.Thk.of Base Ring ea4min=3.76 <= eR=5[mm]	75.1%	OK
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Company Name -

Client :GÜVEN SOGUTMA

Vessel Tag No.:OS.C.130B.35.1

Visual Vessel Design by Hexagon PPM,Ver:20.0 Operator : Rev.:A

EN13445:2014 Issue 5:2018+A5 - EN13445; 16.12 - VERTICAL VESSELS WITH SKIRT

SK.1 Skirt 09 Feb. 2021 14:35 ConnID:S1.1

LOAD CASE NO: 5 - LC6&7&8 OPER.SEISMIC

16.12.6.1 Verification of Membrane Stresses

Membrane Stress(Pint)(Sect.1-1) Sig1pm=88.62 <= fB=288.8[N/mm2]	30.6%	OK
Membrane Stress(Pint)(Sect.1-1) ABS(Sig1qm)=87.34 <= fB=288.8[N/mm2]	30.2%	OK
Membrane Stress(Pext)(Sect.1-1) Sig1pmext=0.4826 <= fB=288.8[N/mm2]	0.1%	OK
Compr.Stress(Pext)(Sect.1-1) Sig1qmext=0.799 <= Sigcall=227.09[N/mm2]	0.3%	OK
Membrane Stress(Sect.2-2) ABS(Sig2pm)=88.16 <= fB=288.8[N/mm2]	30.5%	OK
Membrane Stress(Sect.3-3) ABS(Sig3pm)=1.17 <= fz=111.62[N/mm2]	1.0%	OK
Membrane Stress(Sect.3-3) ABS(Sig3qm)=2.11 <= fz=111.62[N/mm2]	1.8%	OK

16.12.6.2 Bending Stresses

1) Location p ABS(Mp)/Wp=4.46 <= fT=111.62[N/mm2] (16.12-44)	3.9%	OK
2) Location q ABS(Mq)/Wq=8. <= fT=111.62[N/mm2] (16.12-45)	7.1%	OK

16.12.7 Verification of Skirt (Section 4-4)

Membrane Stress(Sect.4-4/z=-60) Sig4pm=1.35 <= fz=111.62[N/mm2]	1.2%	OK
Membrane Stress(Sect.4-4/z=-60) Sig4qm=2.28 <= fz=111.62[N/mm2]	2.0%	OK
Compr.Stress Limits (4-4/z=-60) Sig4c=0.467 <= Sigcall=175.61[N/mm2]	0.2%	OK

Check of Skirt Stability/Weakening due to Openings in Skirt

Stability/Openings in skirt(Sect.4-4/z=-60) StabilityFactor=0.0127 <= 1=1	1.2%	OK
Membrane Stress(Sect.4-4/z=-20) Sig4pm=1.22 <= fz=111.62[N/mm2]	1.0%	OK
Membrane Stress(Sect.4-4/z=-20) Sig4qm=2.15 <= fz=111.62[N/mm2]	1.9%	OK
Compr.Stress Limits (4-4/z=-20) Sig4c=0.467 <= Sigcall=175.61[N/mm2]	0.2%	OK

Check of Skirt Stability/Weakening due to Openings in Skirt

Stability/Openings in skirt(Sect.4-4/z=-20) StabilityFactor=0.012 <= 1=1	1.1%	OK
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Company Name -

Client :GÜVEN SOGUTMA

Vessel Tag No.:OS.C.130B.35.1

Visual Vessel Design by Hexagon PPM,Ver:20.0 Operator : Rev.:A

EN13445:2014 Issue 5:2018+A5 - EN13445; 16.12 - VERTICAL VESSELS WITH SKIRT

SK.1 Skirt 09 Feb. 2021 14:35 ConnID:S1.1

SKIRT BASE DESIGN

Anchor Bolt Area $A_{min}=0.9497 \leq A_e=36.6[\text{mm}^2]$	2.5%	OK
Foundation Bearing Pressure(concrete) $\text{Sigc}=0.6052 \leq f_{cd}/1.35=5.48[\text{N}/\text{mm}^2]$ (16.12-86)	11.0%	OK
Required minimum analysis thickness of bearing plate (FB > 0) $ea_{4min} = \text{MAX}(b_2 * \text{SQR}(3 * \text{SigC}/f_4), \text{SQR}(4 * n * \text{FB} * b_3 / (\text{PI} * D_3 * f_4)))$ (16.12-90a) $= \text{MAX}(36.5 * \text{SQR}(3 * 0.6052 / 203.3), \text{SQR}(4 * 4 * 207.21 * 23 / (3.14 * 50 * 203.3))) = 3.4493 \text{ mm}$		
Min.Thk.of Base Ring $ea_{4min}=3.45 \leq e_R=5[\text{mm}]$	68.9%	OK

Warning: Incorrect location along z-axis of opening in skirt for: InspectionOpening

Volume:0.00 m3 Weight:1.4 kg (SG= 7.849999)

Company Name -

Client :GÜVEN SOGUTMA

Vessel Tag No.:OS.C.130B.35.1

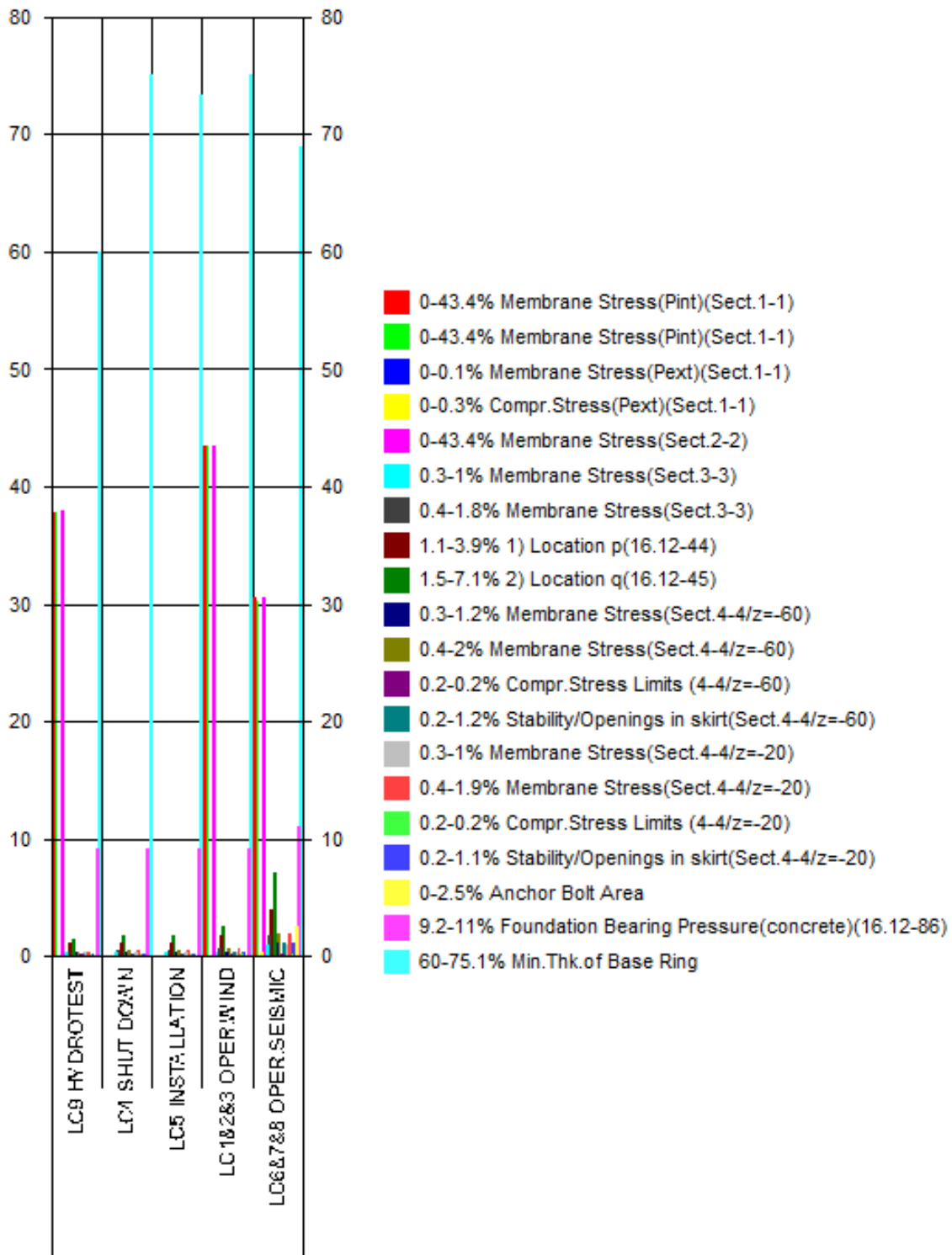
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EN13445:2014 Issue 5:2018+A5 - EN13445; 16.12 - VERTICAL VESSELS WITH SKIRT

SK.1 Skirt

09 Feb. 2021 14:35 ConnID:S1.1

UTILIZATION CHART - SK.1 SKIRT



Max.Utilization/Condition 75.1% CASE:LC4 SHUT DOWN

Company Name -

Client :GÜVEN SOGUTMA

Vessel Tag No.:OS.C.130B.35.1

Visual Vessel Design by Hexagon PPM,Ver:20.0 Operator : Rev.:A

EN13445:2014 Issue 5:2018+A5 - 16.7 - LIFTING LUGS

LL.1 Lifting Lug 14 Jan. 2021 14:03 ConnID:S1.1

INPUT DATA

COMPONENT ATTACHMENT/LOCATION

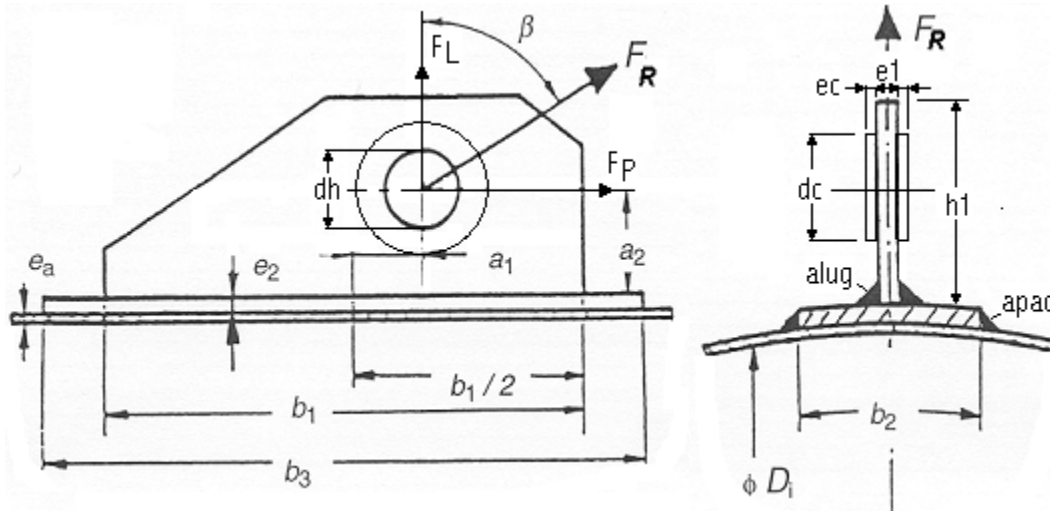
Attachment: S1.1 Cylindrical Shell Shell
z-location of Lug at Loc.of Load Interaction.....:z 550.00 mm
Angular rotation of lug.....:angle 180.00 degr.
Extent of Analysis: Check Lug and Loads in Shell
Load Orientation: Longitudinal Line Load
Type of Lifting Lug:
Symmetric lug with hole in center(a1=0), lift angle -90 to +90 degr.
Design Standard: DNV Cert.Notes 2.7-1 Annex D

SHELL DATA (S1.1)

Shell Type: Cylindrical Shell
OUTSIDE DIAMETER OF SHELL.....:De 168.30 mm
NOMINAL WALL THICKNESS (uncorroded).....:en 7.1000 mm
NEGATIVE TOLERANCE/THINNING ALLOWANCE.....:th 0.7100 mm
INTERNAL CORROSION ALLOWANCE.....:c 0.5000 mm
EN 10217-3:2002/A1:05, 1.0565 P355NH welded tube, HT:N THK<=20mm 100'C
Rm=490 Rp=355 Rpt=304 f=202.67 f20=204.17 ftest=338.1 E=206067(N/mm2) ro=7.85

DATA FOR LIFTING LUG

Cheek Plates/Pad Eyes: Excluded
EN 10025-2:2005, 1.0045 S355JR Flat/Long Products THK<=16mm 50'C
Rm=470 Rp=355 Rpt=355 f=195.83 f20=195.83 ftest=338.1 (N/mm2)
NOTE: A PARTICULAR MATERIAL APPRAISAL(PMA) MAY BE REQUIRED FOR THIS MATERIAL.
Comment:
Exclude shell corrosion, only use lifting lug in uncorroded condition.: NO
LENGTH OF LIFTING LUG AT SHELL/PAD JUNCTION.....:b1 40.00 mm
HEIGHT OF LIFTING LUG.....:h1 50.00 mm
THICKNESS OF LIFTING LUG.....:e1 5.0000 mm
DIAMETER OF HOLE IN LIFTING LUG.....:dh 30.00 mm
DISTANCE FROM LOAD TO SHELL OR REINFORCEMENT PAD....:a2 30.00 mm



DATA FOR REINFORCEMENT PAD

Reinforcement Pad: Excluded

WELDING DATA

Type of Weld - Lug to Pad/Shell: Double Sided Fillet Weld
WELD JOINT COEFFICIENT.....:z 0.8500
WELD BETWEEN LUG AND PAD/SHELL, THROAT DIMENSION....:alug 2.0000 mm

Company Name -

Client :GÜVEN SOGUTMA

Vessel Tag No.:OS.C.130B.35.1

Visual Vessel Design by Hexagon PPM,Ver:20.0 Operator : Rev.:A

EN13445:2014 Issue 5:2018+A5 - 16.7 - LIFTING LUGS

LL.1 Lifting Lug 14 Jan. 2021 14:03 ConnID:S1.1

LOAD DATA

Load Description	ID	Units	Load Case 1
Pressure	P	MPa	0
Not Applicable			0.5
Test Condition (Yes/No)			No
Temp.D=Design/A=Ambient	Temp		A
Maximum Force on Lug (at angle Beta)	FR	kN	0.6
Angle of Sling Leg From Vertical	Beta	degr.	45
Load Safety Factor	SL		1.5
Percentage Skew/Side Load	PS	%	0

Analyse Lifting Loads for Horizontal to Vertical Rotational Lift.: NO

WELDING REQUIREMENTS TO EN 1708-1:2010

Comment(Optional):

Type of welded connection: Not Applicable

CALCULATION DATA

PRELIMINARY CALCULATIONS

Shell Analysis Thickness ea

$$ea = en - c - th = 7.1 - 0.5 - 0.71 = 5.8900 \text{ mm}$$

Shell Inside Diameter

$$Di = De - 2 * (en - c) = 168.3 - 2 * (7.1 - 0.5) = 155.10 \text{ mm}$$

16.6.3 Equivalent Shell Diameter

$$Deq = Di (16.6-1) = 155.1 = 155.10 \text{ mm}$$

$$\text{Lambda} = b / \text{SQR}(Deq * ea) (16.6-13/17) = 40 / \text{SQR}(155.1 * 5.89) = 1.3234$$

16.7.3 CONDITIONS OF APPLICABILITY

$$\text{»a) } 0.001 = 0.001 \leq en / Deq = 0.0458 \ll \text{ OK} \ll$$

$$\text{»a) } en / Deq = 0.0458 \leq 0.05 \ll \text{ OK} \ll$$

LOAD CASE NO: 1 - LOAD CASE 1

$$K2 \text{ (design condition)} = == 1.2500$$

Normal Force Component

$$FL = SL * FR * \text{Cos}(\text{beta}) = 1.5 * 600 * \text{Cos}(45) = 0.6364 \text{ kN}$$

Parallel Force Component

$$FP = SL * FR * \text{Sin}(\text{beta}) = 1.5 * 600 * \text{Sin}(45) = 0.6364 \text{ kN}$$

Side/Skew Load - 0% Lateral Load

$$Fside = PS / 100 * SL * FR = 0 / 100 * 1.5 * 600 = 0.00 \text{ kN}$$

External Moment Along Load Direction

$$ML = SL * FR * ((a2 + e2) * \text{Sin}(\text{beta}) - a1 * \text{Cos}(\text{beta})) \\ = 1.5 * 600 * ((30 + 0) * \text{Sin}(45) - 0 * \text{Cos}(45)) = 0.0191 \text{ kNm}$$

External Moment in Transverse Load Direction

$$MT = Fside * (a2 + e2) = 0 * (30 + 0) = 0.00 \text{ kNm}$$

Stresses in the Lug Foot/Across Baseline and at Weld

Tensional Stress in the Lug Foot

$$\text{SigTension} = FL / (e1 * b1) = 636.4 / (5 * 40) = 3.1820 \text{ N/mm}^2$$

Bending Stress due to FP

$$\text{SigBendL} = 6 * FP * a2 / (e1 * b1^2) \\ = 6 * 636.4 * 30 / (5 * 40^2) = 14.32 \text{ N/mm}^2$$

Bending Stress in Lug Plate due to Moment in Transverse Load Direction

$$\text{SigBendT} = 6 * Fside * a2 / (b1 * e1^2) \\ = 6 * 0 * 30 / (40 * 5^2) = 0.00 \text{ N/mm}^2$$

Shear Stress due to FP

$$\text{TauL} = FP / (b1 * e1) = 636.4 / (40 * 5) = 3.1820 \text{ N/mm}^2$$

Shear Stress in Transverse Load Direction

$$\text{TauT} = Fside / (b1 * e1) = 0 / (40 * 5) = 0.00 \text{ N/mm}^2$$

Effective Stress

$$\text{Sige} = \text{SQR}((\text{SigTension} + \text{SigBendL} + \text{SigBendT})^2 + 3 * (\text{TauL}^2 + \text{TauT}^2)) \\ = \text{SQR}((3.18 + 14.32 + 0)^2 + 3 * (3.18^2 + 0^2)) = 18.35 \text{ N/mm}^2$$

Company Name -

Client :GÜVEN SOGUTMA Vessel Tag No.:OS.C.130B.35.1

Visual Vessel Design by Hexagon PPM,Ver:20.0 Operator : Rev.:A

EN13445:2014 Issue 5:2018+A5 - 16.7 - LIFTING LUGS

LL.1 Lifting Lug 14 Jan. 2021 14:03 ConnID:S1.1

Effective Stress in the Lug Foot $\sigma = 18.35 \leq$ $f_l = 195.83 [N/mm^2]$	9.3%	OK
Bending Stress in Shell due to Moment in Transverse Load Direction $\sigma_{BendT2} = 6 * F_{side} * a_2 / (b_1 * e_a^2)$ $= 6 * 0 * 30 / (40 * 5.89^2) = 0.00 \text{ N/mm}^2$		
Bending Stress in Shell/Pad(Transverse Moment) $\sigma_{BendT2} = 0 \leq 1.5 * f_s = 306.25 [N/mm^2]$	0.0%	OK

Double Fillet Weld between Lug and Pad/Shell(Bednar Chapter 10.3)

Weld Length
 $L_w = 2 * (e_1 + b_1) = 2 * (5 + 40) = 90.00 \text{ mm}$
 Section Modulus(around axis transverse to lug)
 $Z_x = e_1 * b_1 + b_1^2 / 3 = 5 * 40 + 40^2 / 3 = 733.33 \text{ mm}^2$
 Section Modulus(around axis along lug)
 $Z_y = b_1 * e_1 + e_1^2 / 3 = 40 * 5 + 5^2 / 3 = 208.33 \text{ mm}^2$
 Unit force due to FL
 $f_1 = F_L / L_w = 636.4 / 90 = 7.0711 \text{ N/mm}$
 Unit force due to FP and Fside
 $f_2 = \sqrt{(F_P^2 + F_{side}^2)} / L_w = \sqrt{(636.4^2 + 0^2)} / 90 = 7.0711 \text{ N/mm}$
 Bending
 $f_3 = \text{MAX}((F_P * a_2 - F_L * a_1) / Z_x, F_{side} * a_2 / Z_y)$
 $= \text{MAX}((636.4 * 30 - 636.4 * 0) / 733.33, 0 * 30 / 208.33) = 26.03 \text{ N/mm}$
 Resultant Load
 $f_{tot} = \sqrt{(f_1 + f_3)^2 + f_2^2} = \sqrt{(7.07 + 26.03)^2 + 7.07^2} = 33.85 \text{ N/mm}$
 Required Weld Size, Throat Dimension
 $a_{lugmin} = f_{tot} / (z * f_s) = 33.85 / (0.85 * 204.17) = 0.1951 \text{ mm}$

Required Lug Weld Size $a_{lugmin} = 0.1951 \leq a_{lug} = 2 [mm]$	9.7%	OK
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Tear Out Stress , DNV Cert.Notes 2.7-1 Annex D: 2017

$\tau_{TearOut} = 3 * S_L * F_R / (e_1 * 2 * (h_1 - a_2 - d_h / 2))$
 $= 3 * 1.5 * 600 / (5 * 2 * (50 - 30 - 30 / 2)) = 54.00 \text{ N/mm}^2$

Tear Out Stress $\tau_{TearOut} = 54 \leq R_e(lug) = 355 [N/mm^2]$	15.2%	OK
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Contact/Bearing Stress (Pin in Hole) DNV Cert.Notes 2.7-1 Annex D: 2017

Note: Formula for compressive stress assumes a maximum difference in diameterspin/hole of 6%.
 $\sigma_{Bearing} = 23.7 * \sqrt{S_L * F_R / (e_1 * d_h)}$
 $= 23.7 * \sqrt{1.5 * 600 / (5 * 30)} = 58.05 \text{ N/mm}^2$

Bearing Stress(pin in hole) $\sigma_{Bearing} = 58.05 \leq R_e(lug) = 355 [N/mm^2]$	16.3%	OK
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16.6.7 - Global Circumferential Membrane Stress

$\sigma_{gmy} = P * D_{eq} / (2 * e_a) (16.6-11/12) = 0 * 155.1 / (2 * 5.89) = 0.00 \text{ N/mm}^2$

16.6.8 - Single Line Loads

$K_{13} = 1 / (1.2 * \sqrt{1 + 0.06 * \lambda^2}) (16.6-15) = 1 / (1.2 * \sqrt{1 + 0.06 * 1.32^2}) = 0.7927$
 $K_{14} = 1 / (0.6 * \sqrt{1 + 0.03 * \lambda^2}) (16.6-16) = 1 / (0.6 * \sqrt{1 + 0.03 * 1.32^2}) = 1.6245$
 $N_{y1} = \text{MIN}(0.08 * \lambda, 0.2) (16.6-14) = \text{MIN}(0.08 * 1.32, 0.2) = 0.1059$
 $N_{y2} = \sigma_{gmy} / (K_2 * f_s) (16.6-8) = 0 / (1.25 * 204.17) = 0.00$
 $K_1 \text{ from figure 16.6-1} = 1.464$
 Bending Limit Stress σ_{ball}
 $\sigma_{ball} = K_1 * K_2 * f_s (16.6-6) = 1.464 * 1.25 * 204.17 = 373.62 \text{ N/mm}^2$

Maximum Allowable Local Force F_{Rmax}

$F_{Rmax} = \sigma_{ball} * e_a^2 / (K_{13} * \text{Abs}(\cos(\beta)) + K_{14} * \text{Abs}(a_2 * \sin(\beta) - a_1 * \cos(\beta))) / b_1$

14 LL.1 Lifting Lugs Lifting Lug	$U_{max} = 16.3\%$	Page: 74
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Company Name -

Client :GÜVEN SOGUTMA

Vessel Tag No.:OS.C.130B.35.1

Visual Vessel Design by Hexagon PPM,Ver:20.0 Operator : Rev.:A

EN13445:2014 Issue 5:2018+A5 - 16.7 - LIFTING LUGS

LL.1 Lifting Lug 14 Jan. 2021 14:03 ConnID:S1.1

$$\frac{(16.7-4) \cdot 373.62 \cdot 5.89^2}{(0.7927 \cdot \text{Abs}(\text{Cos}(0.7854)) + 1.62 \cdot \text{Abs}(30 \cdot \text{Sin}(0.7854) - 0 \cdot \text{Cos}(0.7854))) / 40} = 9.1146 \text{ kN}$$

Local Force on Lifting Lug(Long.Direction) FR=0.6 <= FRmax=9.11[kN]	6.5%	OK
Local Force on Lifting Lug(Long.Direction) SL*FR=0.9 <= FRmax*(Sigballt/Sigball)=15.09[kN]	5.9%	OK

CALCULATION SUMMARY**LOAD CASE NO: 1 - LOAD CASE 1**

Effective Stress in the Lug Foot Sige=18.35 <= fl=195.83[N/mm2]	9.3%	OK
Bending Stress in Shell/Pad(Transverse Moment) SigBendT2=0 <= 1.5 * fs=306.25[N/mm2]	0.0%	OK
Required Lug Weld Size alugmin=0.1951 <= alug=2[mm]	9.7%	OK
Tear Out Stress TauTearOut=54 <= Re(lug)=355[N/mm2]	15.2%	OK
Bearing Stress(pin in hole) SigBearing=58.05 <= Re(lug)=355[N/mm2]	16.3%	OK
Local Force on Lifting Lug(Long.Direction) FR=0.6 <= FRmax=9.11[kN]	6.5%	OK
Local Force on Lifting Lug(Long.Direction) SL*FR=0.9 <= FRmax*(Sigballt/Sigball)=15.09[kN]	5.9%	OK

Volume:0.00 m3 Weight:0.1 kg (SG= 7.85)

Company Name -

Client :GÜVEN SOGUTMA

Vessel Tag No.:OS.C.130B.35.1

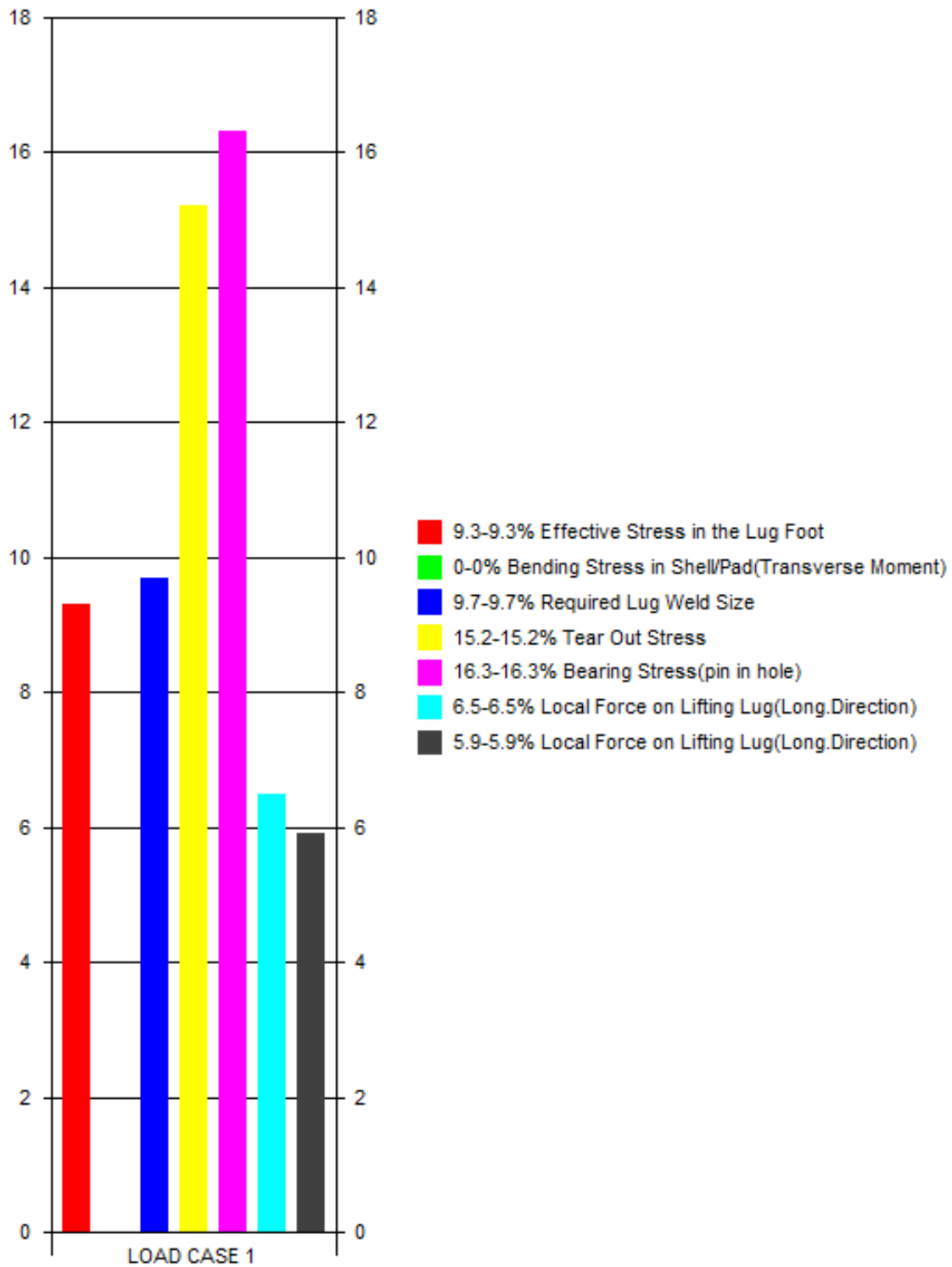
Visual Vessel Design by Hexagon PPM,Ver:20.0 Operator : Rev.:A

EN13445:2014 Issue 5:2018+A5 - 16.7 - LIFTING LUGS

LL.1 Lifting Lug

14 Jan. 2021 14:03 ConnID:S1.1

UTILIZATION CHART - LL.1 LIFTING LUG



Max.Utilization/Condition 16.3% CASE:LOAD CASE 1